

**GEOTECHNICAL SITE CHARACTERIZATION
PORT OF ROCHESTER HARBOR IMPROVEMENT
AND HARBOR FERRY TERMINAL
ROCHESTER, NEW YORK**

Draft

by

**Haley & Aldrich of New York
Rochester, New York**

for

**LaBella Associates, P.C.
Rochester, New York**

**File No. 70819-000
September 2000**

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LaBella Associates, P.C.
300 State Street
Rochester, New York 14614

Attention: Sergio Esteban, P.E.

Subject: Port of Rochester Harbor Improvement and Harbor Ferry Terminal
Rochester, New York

Ladies and Gentlemen:

We are pleased to submit herewith our report entitled, *Geotechnical Site Characterization, Port of Rochester Harbor Improvement and Harbor Ferry Terminal, Rochester, New York*. The work was undertaken at your request, as outlined in our proposal dated 4 June 1999 and authorized under our Subconsultant Agreement, dated 16 December 1999.

This report presents a compilation of the results of historic and new subsurface explorations, field testing, laboratory testing, groundwater observations, and site geotechnical engineering interpretations pertinent to the planning and preliminary design of the proposed ferry terminal and related infrastructure.

If additional information regarding the data or conclusions presented in this report is required, please do not hesitate to contact us. It has been a pleasure working with you and the other project team members on this exciting project, and we look forward to our continued association during subsequent phases of the project.

Sincerely yours,

HALEY & ALDRICH OF NEW YORK

Maureen S. Valentine, P.E.
Senior Engineer

Stanley E. Walker, P.E.
Vice President

Enclosures

c: Bourne Consulting Engineers, Attn: Ronald Bourne, P.E.

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EXECUTIVE SUMMARY

This report presents a summary of research, exploration, and characterization of the subsurface conditions at the site of the proposed Port of Rochester Harbor Improvement and Harbor Ferry Terminal, City of Rochester Project ID # 99021, conducted by Haley & Aldrich of New York. It has been prepared in accordance with our Subconsultant Agreement with LaBella Associates, P.C. dated 16 December 1999.

The purpose of this study was to characterize the site's subsurface conditions in sufficient detail to support the planning and preliminary design of the proposed site improvements.

This report contains reproductions of historic (Sanborn) maps (1892 to 1967) depicting the various facilities that have occupied the site and records of several earlier subsurface explorations made on or near the site. It also contains detailed records of the 25 test borings, 27 test pits, and 3 groundwater observation wells installed as part of the current study of the site by Haley & Aldrich, LaBella Associates, and Bourne Consulting Engineers.

The project area has undergone significant geologic and man-made alteration. An approximately 85-foot-deep, steep-sided gorge in the bedrock formed by post-glacial erosion, encroaches beneath the former transit sheds (the North and South Warehouses) along the eastern edge of the site. From the gorge's edge the bedrock surface rises more gently from about Elev. 200 (City Datum) to Elev. 215 to 235 near the western edge and southern end of the site, ranging from about 60 to 20 feet below the present ground surface. The much of the site is underlain by man-placed fill consisting of uncontrolled deposits of soil and iron-manufacturing slag and demolition rubble ranging from as much as 20 feet to as little as 1 foot in thickness. The fill varies quite randomly from loose to dense. In most areas loose alluvial (river-deposited) fine sand and silt underlie the fills which extend to depths of a few to more than 100 feet. Groundwater levels appear to be about 2 to 5 feet above river level.

These conditions, while providing generally fair support for at-grade roadways and parking areas, provide variably fair to poor support for buildings and additional earthfills. The loose fills and alluvial deposits could yield detrimental differential settlements under thick regrading fills and moderately to heavily loaded structures.

Careful consideration should be given to the existing data presented in this report and the need for additional exploration, testing, and evaluation of the subsurface conditions in the planning and design of any proposed site and structural improvements.

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I. INTRODUCTION

1.01 Purpose

This geotechnical report has been prepared to assist planning and preliminary engineering efforts for the proposed Port of Rochester Harbor Improvements and Harbor Ferry Terminal, in Rochester, New York.

Haley & Aldrich of New York (H&A) was retained by LaBella Associates, P.C. and the City of Rochester to collect and assimilate existing and new geotechnical and geologic information pertinent to the project to provide a general characterization of subsurface conditions at the project location. This document represents a collection of data, some developed by H&A in the past and some by others, that establish the general regional conditions. Site-specific subsurface explorations and field and laboratory testing were performed as part of the site characterization study.

This report is preliminary in nature, given the state of development for the project. The scope of the investigations has been to collect, assemble and interpret site and subsurface information in order to develop an understanding of the regional subsurface conditions, sufficient to complete initial planning efforts and preliminary engineering design. Additional detailed design phase geotechnical investigations will likely be required to more fully evaluate the significance of the subsurface conditions to the design and long-term performance of the project elements.

1.02 Project Description

The proposed Port of Rochester Harbor Improvements site is located as shown on Figure 1 - Project Locus, and is bounded to the east by the Genesee River, to the north by Beach Avenue, to the west by Lake Avenue and to the south by the Stutson Street Bridge. Currently, the site is occupied by parking facilities and two existing warehouses, a boat ramp, an excursion ferry dock, several privately operated marinas and commercial establishments. A CSX railroad crosses the central portion of the project area with a branch to the south along the western bank of the river. Approximate locations of existing structures are shown on Figure 2 - Exploration Location Plan.

This project consists of the preliminary design of proposed transportation access, building facilities and waterside improvements associated with the Port of Rochester Harbor Improvement. The proposed improvements are intended to expand and enhance public access to the waterfront as well as provide the necessary infrastructure to support public recreation, transportation and economic development opportunities. According the contract documents, the project scope includes, but it not limited to the following:

➤ Access/Transportation

- Reconstruction of existing street pavement and sidewalks;
- Construction of new streets and sidewalks;
- Improvement of existing and construction of new parking facilities;
- Riverfront pedestrian promenade;
- Streetscape and site landscape features;
- Street and site lighting;
- Signage and graphics;
- Public and private utility improvements.

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- Building Facilities
 - New building for a ferry terminal and border crossing operation;
 - Kiosks and marina support structures;
 - Rehabilitation of the north warehouse;
 - Signage and graphics;
 - Public and private utility services.
- Waterside Improvements
 - Transient marina and related facilities;
 - Ferry boat and excursion vessel berthing facilities;
 - River wall rehabilitation and/or reconstruction;
 - Navigation improvements;
 - Marina extension along River Street to Petten Street;
 - Public and private utility services and fueling facilities.

At the time of this writing, the proposed improvements are in the conceptual stage. Several alternate configurations are being considered for design.

1.03 Elevation datum

Elevations used herein are referenced to the City of Rochester Datum. Historical elevations are shown in the Appendices as referenced by the original project, but have been adjusted to the City of Rochester Datum within the tables and text.

II. FIELD AND LABORATORY INVESTIGATIONS

2.01 Background Information

Several earlier investigations in the general area of the project have been conducted by H&A and others, as listed below. The locations of the explorations associated with these earlier investigations are shown on Figure 2, Exploration Location Plan. A summary of the conditions encountered by these explorations is presented in Table II - Conditions Encountered in Earlier Subsurface Investigations:

- Stutson Street Water Main - Genesee River Crossing: Rochester Drilling Company between 13 and 21 September 1989 performed explorations, under the observation of H&A of New York. The work was performed for Joseph C. Lu, P.E for the design of a force main crossing the Genesee River.
- Stutson Street Bridge: Explorations were performed between 2 November 1994 and 11 December 1997, under the observation of NYSDOT personnel, for the design of a replacement for the Stutson Street Bridge.
- Army Corps of Engineers Dredge Probes: the Army Corps of Engineers performed Probe explorations between February 1959 and April 1961 for a Rochester Harbor Deepening project.
- Wave Surge Protection Project: the Army Corps of Engineers performed drive sample explorations in December 1960 for a Wave Surge Protection Project in the Rochester Harbor.
- Rehabilitation of East Pier: Explorations were performed by Empire Soils Investigation Inc. between April 1973 and August 1985 under the observation of Army Corps of Engineers personnel for a design analysis of the East Pier Repair in the Rochester Harbor.
- Lake Avenue Improvements: Explorations were performed by Vanderhorst between 16 and 17 March 1999 for Bergmann Associates, P.C. and the City of Rochester for the planning and preliminary engineering of the Lake Avenue Improvement Project.

Copies of the logs of the explorations made during these investigations are presented in Appendix C.

2.02 Recent Investigations

Three sets of objective-specific explorations were made as a part of this site characterization study. A summary of the conditions encountered in each of the explorations is presented in Table I – Conditions Encountered in Recent (2000) Subsurface Investigations.

In mid-January, 2000, four backhoe-dug test pits were made under the direction of Bourne Consulting Engineers, to explore the configuration and condition of the existing quay wall and its tieback anchorage system. These test pits were observed and logged by Haley & Aldrich. The locations of these test pits, designated BCE-TP # 1 through #4, are shown on Figure 2. Field logs of these explorations are presented in Appendix A-1.

In late-February, 2000, twenty-two backhoe-dug test pits were made under the direction of LaBella Associates, to explore the physical and chemical character of the near-surface subgrade materials and the groundwater levels at the site. LaBella Associates observed, logged, and sampled the test pits. A Haley & Aldrich representative observed and viewed samples from

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several of these test pits. The locations of these test pits, designated LBA-TP #1 through #22, as surveyed by LaBella Associates, are shown on Figure 2. The logs of these test pits are presented in Appendix A-2.

Between 23 May and 13 June 2000, twenty-four test borings, HA-101 through HA-107, and HA-109 through HA-125, were drilled by Geologic Enterprises, Inc., of Cortland, New York, at locations selected by Haley & Aldrich to aid in characterizing the soil and bedrock conditions at the site. Tests boring locations and ground surface elevations, as shown on Figure 2, were determined by LaBella Associates.

The borings were drilled using hollow-stem augers to depths below ground surface ranging from 10 to 116 ft. Soil samples were recovered continuously within the fill and at 5-ft intervals thereafter by driving a 1 $\frac{3}{8}$ -in. I.D. split-spoon sampler with a 140-lb. hammer consistent with ASTM Method D1586. The "N" value was determined at each sample interval by counting the number of blows required to drive the split-spoon sampler a distance of 24 in. below the bottom of the hollow stem auger and into the soil under the impact of the hammer free-falling 30 inches. The "N" value is taken as the number of blows required to advance the sampler from 6 to 18 inches within the 24-inch sample range. A Haley & Aldrich geologist monitored the drilling and logged the recovered soil samples.

Bedrock was cored in five of the borings, HA-102, -107, -111, -121, and -122, using an NX-size (1-7/8 in. I.D.) corebarrel. Bedrock was cored to depths ranging from 2.0 to 10.0 ft.

Test Boring and Core Boring Reports prepared by Haley & Aldrich are presented in Appendix A-3. It should be noted that boring reports and related information depict subsurface conditions and water levels at the specific locations at the time of drilling. Soil conditions at other locations may differ from conditions encountered in the explorations. Groundwater conditions at any of the exploration locations may also change with time.

2.03 Groundwater Observation Wells

Groundwater observation wells were installed in completed boreholes HA-111 (MW-1, HA-114a (MW-3), and HA-117 (MW-2). The wells consisted of 2-inch-diameter, perforated PVC screen placed at or below groundwater level, and a solid PVC riser extending to approximately 2 ft. above ground surface. The annulus between the PVC pipe and the borehole was backfilled with filter sand, and bentonite seals were placed above the PVC screen.

Observation Well Installation Reports prepared by Haley & Aldrich for each of these wells are included Appendix A-4.

2.04 Laboratory Soil Testing

Ray M. Teeter, P.E. of Fairport, New York, performed laboratory tests on six soil samples from the test borings. The soil testing consisted of sieve analyses and hydrometer tests to quantify the grain-size distribution of the soils, and Atterberg limits and moisture content determinations to assess the plasticity of the fine-grained soils. The results of these tests have been incorporated into the soil descriptions shown on the Test Boring Reports, and are presented in Appendix B, Table B-1. Soil testing data from earlier investigations are presented in Table B-2

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III. SITE AND SUBSURFACE CONDITIONS

3.01 Site conditions

The project site is located on the western side of the Genesee River, at its discharge to Lake Ontario. The project site has been part of extensive planning over the years, and includes the area bounded by Beach Avenue to the north, Lake Avenue to the west, and the Genesee River to the east. The south end of the project extends beyond the Stutson Street Bridge.

The area north of the CSX railroad is currently occupied by parking facilities for Charlotte Beach, a boat ramp and two existing warehouse structures along the river walk. The foundation from a third warehouse structure remains visible. A group of municipal buildings occupies the southwest corner, near Lake Avenue. This portion of the project slopes downward to the northeast from Lake Avenue toward the Genesee River. The ground surface elevations range from approximately El. 290 near the Lake Avenue crossing of the railroad, to El. 250 in the northeast corner, beyond the existing warehouse structures. Historically, this portion of the project area has housed an iron works which changed hands several times and became a steel company, associated rail lines, a rail loop turnaround, a ball park and yacht club, a steam boat wharf, later boat ramps, three warehouses, and various configurations of roads and parking facilities.

The project area south of the CSX railroad, between River Street and the river, is currently occupied by residential structures, boat docks, boat storage yards, and a small water treatment facility. This portion of the site is relatively level, with elevation ranging from El. 252 to 254 between River Street and the Genesee River. The ground surface slopes up relatively steeply at the railroad embankment, to an elevation of El. 260. West of River Road, the ground surface also slopes up relatively steeply to a crest at approximately El. 283 at the lighthouse. Historically, this portion of the project has housed a planing mill, which later became a veneer works and boat manufacturing facility, various boat-docking facilities and associated structures.

Historic (Sanborn) maps depicting on-site structures and facilities in 1892, 1912, 1924, 1950, And 1967 superimposed on the current site mapping are presented in Appendix D.

3.02 Subsurface soil and rock conditions

A. Geologic History

Rochester lies within the relatively low and flat-lying physiographic province known as the Erie-Ontario Lowland, which begins at Lake Ontario and extends southward to the Appalachian Plateau. The Genesee River runs north-south through the Erie-Ontario Lowland.

Bedrock underlying the northern portion of Rochester is part of the Queenston Formation, which is exposed in outcrops of the Genesee River gorge from Lake Ontario to the Driving Park Avenue Bridge at the Lower Falls. The Queenston Formation is an approximately 1000-foot thick sequence of alternating, nearly level shale and sandstone beds of Ordovician age, formed in deep seawater.

As a result of land movements associated with the building of the Appalachian Mountains to the south and east, the land surface of western New York was raised above the inland seas.

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Subsequently, the exposed rocks were tilted southward and a southerly drainage system developed along the shallow dipping beds.

A long period (about 350 million years) of erosion ensued until glaciation occurred during the Pleistocene Epoch. It is estimated that over 2000 feet of rock was stripped from the region during this time.

Major streams, which formed during this erosional period, were the ancient Ontarian River and the Genesee River, which flowed into the Ontarian River at what is now Irondequoit Bay.

Approximately 3.0 to 1.5 million years ago, glacial scouring deepened the Ontarian River Valley, which, upon retreat of the glaciers, became present-day Lake Ontario. Glacial scouring also deepened the pre-glacial Genesee River Valley. As the ice retreated during the past 20,000 years, differential post-glacial uplift resulted in flooding of the pre-glacial Genesee River Valley, forming present day Irondequoit Bay. The Genesee River then established its present course by eroding a new deep channel in the exposed bedrock some 4 to 5 miles west of the pre-glacial Genesee Valley.

Fairchild (1918) concluded in his paper, "The Rochester Canyon and the Genesee River Base-Levels", that the present river valley was formed by post-glacial erosion, the depth of which was regulated by the varying water levels in what is now Lake Ontario. His studies indicated the "canyon" bottom at the lakeshore to be about Elev. 100, about 145 feet below the present lake level.

B. On-site Conditions

Site stratigraphy was evaluated on the basis of the findings of the test borings, test pits and readily available public information regarding the local geology and hydrology. The borings encountered three principal soil units at the site; fill, alluvial sediments and glacial till. Generalized descriptions of the soil units and encountered thicknesses are presented below.

FILL – Man-placed fill materials, ranging from silty sand and gravel to varying combinations of iron-manufacturing waste slag, demolition rubble (bricks, concrete, and railroad ties), remnant concrete slabs and foundations, and some organic matter, in thicknesses ranging from 1 to 20 feet, were encountered in essentially all of the on-site explorations. Standard Penetration Test values (blows to advance the sampler 1 foot) varied erratically from 4 to refusal on impenetrable objects, reflecting the varying and uncontrolled nature of the fill deposits. The estimated bottom-of-fill surface, as inferred by Haley & Aldrich, is depicted on Figure 3 – Bottom-of-Fill Contour Map.

ALLUVIUM – Alluvium (stream-deposited soil) was encountered beneath the topsoil or fill in most all of the on-site borings, extending to depths of a few feet toward the western side of the site to as much as 114 feet below the ground surface in the deep borings (HA- 101 and HA-123) at the river's edge. The alluvial soils consist of silty medium to fine sand with varying amounts of gravel with occasional zones of plastic, slightly organic clayey silt with some fine sand. In some test pits remnants of former surface vegetation were observed directly beneath overlying fill material. The samples ranged from dry to wet, generally increasing in moisture content with depth. Results of grain-size analyses and Atterberg limit and moisture content determinations on samples of the alluvial deposits are presented in Appendix B. Standard Penetration Test values ranged from 0 to more than 50 blows per foot and averaged from 3 to more than 20 in

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individual borings, indicating the generally loose to very loose condition of these river-deposited sediments.

GLACIO-LACUSTRINE DEPOSITS – Deposits of late-glacial lakebed sediments consisting of stratified fine sands with occasional clay and coarser sand layers were encountered in thicknesses of up to 10 feet overlying glacial till in several explorations in the higher ground toward Lake Avenue.

GLACIAL TILL - Glacial till was encountered directly below the fill or alluvial or lacustrine sediments and extended to the top of the bedrock in most of the borings. In a few borings, HA-101, -109, -110, and -123, the glacial till was missing and the alluvium extended directly to bedrock. The till materials encountered ranged from soft to hard sandy, silty clay with trace gravel or clayey silt with sand and fine gravel. However, in general the undisturbed till was found to be very compact. The samples ranged from dry to moist. The estimated top-of-till surface, as inferred by Haley & Aldrich, is depicted on Figure 4 – Top-of-Till Contour Map.

A mixture of rock fragments and soil, identified as weathered bedrock, was encountered in a few of the borings. Visual descriptions ranged from “very dense red brown silty fine to coarse SAND, trace clay” to “disintegrated red sandstone”. Borings HA-102, -109, -110, -122, and -123 penetrated weathered bedrock, encountering thicknesses of 1.0 to 5.0 ft.

Bedrock cored in the explorations consisted of relatively flat-lying sedimentary rocks of the Queenston Formation. This unit is described individually below:

SANDSTONE – A relatively massive layer of sandstone of the Queenston formation was encountered beneath the alluvium and glacial till at depths ranging from 27 to 114 ft. below the ground surface. The core samples recovered from the test borings are described as moderately weathered to competent red, fine-grained sandstone with interbedded or mottled gray sandstone. RQD values ranged from 38 to 69 percent, indicating that the quality is pore to fair.

The bedrock surface was encountered at elevations ranging from El. 138 (auger refusal in HA-101) and El. 139 (HA-123) to El. 232 (in HA-110).

However, earlier borings (DN-B-51 and -B-52) made in the river south of the present Stutson Street Bridge’s east abutment, did not encounter bedrock or refusal before reaching elevations at or below 118. These depths are consistent with Fairchild’s (1918) findings and together with the present exploration findings support the inference that there is a deep curvilinear trough in the bedrock passing beneath and to the east of the project site on its course to the deeper water off shore.

The estimated top-of-bedrock surface, including a speculated configuration of this deep “canyon”, as inferred by Haley & Aldrich, is depicted on Figure 5 – Top-of-Bedrock Contour Map.

3.03 Groundwater Conditions

The depth to water was recorded at completion of the explorations in borings HA 102, 107, 111, and 118 at depths ranging from 3.0 to 18 ft. below the existing ground surface. Water levels were also measured in each of the three piezometers at depths ranging from 3.60 to 10.74 ft.

below the existing ground surface. In general, the water table in late-May to early-June 2000 appears to have been between El. 248 and 251, sloping downward from west to east and being 2 to 5 feet above normal river level.

Corps of Engineers' river-level data reviewed and summarized by Bourne Consulting Engineering (5-10-00) indicates a maximum-recorded water level at El. 250.39. The Flood Insurance Rate Map for Rochester shows the project site as "Zone C" - subject to minimal flooding. ^R

Water levels at the site should be expected to vary with precipitation, season, temperature and construction activity in the area. Therefore, groundwater levels during and following construction may differ from those observed in the test borings.

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IV. GEOTECHNICAL ENGINEERING CONSIDERATIONS

The site's geologic and use history have produced subsurface conditions which warrant careful consideration in planning its redevelopment. These conditions include markedly varying bedrock surface elevations; deep, loose, and potentially compressible natural soil deposits; remnant foundations of earlier facilities; extensive areas of filled land containing iron-manufacturing slag and other wastes and demolition rubble; and relatively shallow groundwater levels. Detailed subsurface exploration and testing programs will be needed to establish appropriate design criteria and support construction planning for significant site improvements.

4.01 Design Considerations for Site Infrastructure Improvements

The uncontrolled fills and shallow groundwater that underlie most of the site present variable and potentially settlement-yielding support for streets and parking lots and a possibly corrosive environment for underground utilities. The chemical character and potential corrosivity (to concrete and metals) of the groundwater should be assessed in conjunction with the design of such facilities. The addition of more than 1 or 2 feet of fill to the present grade could cause noticeable, long-term settlements in areas of poorer subgrade conditions. To minimize the impact of post-construction differential settlements, site regrading, preceded by removal of existing topsoil and pavement and thorough proof-rolling of the exposed subgrade with a heavy, smooth-drum, vibratory compactor, should be completed prior to the construction of infrastructure improvements. Subgrade and surface drainage should be carefully developed to assure the long-term performance of trafficked areas. The presence of the loose fills and shallow groundwater should be carefully considered in the planning and execution of all utility trenching and installation.

4.02 Design Considerations for Foundations

The existing uncontrolled fills present widely varying support for foundations and could yield significant general or differential settlements under moderately to heavily loaded foundations. The buried slag and other waste and affected groundwater could pose threats to the long-term integrity of concrete or steel foundations. Removal and replacement or partial removal and insitu densifications of the existing fill materials and replacement with controlled fill may be appropriate for moderately loaded structures. Heavily loaded or settlement-intolerant structures would most likely require deep foundations (piles or caissons) seated on or in the glacial till or bedrock.

4.03 Design Considerations for Below-grade Walls

The shallow groundwater and loose fill and alluvial sediments will exert considerable horizontal loadings on temporary and permanent earth-retaining structures. Chemically aggressive groundwater could pose a threat to the long-term integrity of earth-retaining walls, particularly those constructed of steel. Care must be taken to assure sufficient lateral support both at the top and at or below the bottom of the excavation or below-grade floor.

4.04 Design Groundwater Levels

In view of the levels observed in the recently installed observation wells, the presence of the confining sheet-pile quay wall, and the potential (minimal) for site flooding, design groundwater levels should be taken as the finished ground surface throughout the site.

4.05 Seismic Design Considerations

The site is located within Seismic Zone A of the proposed (1999) Seismic Zoning Map for New York State Seismic Building Code. Zone A has a seismic zone factor, $Z = 0.09$, which numerically corresponds to effective peak acceleration in g on rock /stiff soil S1 conditions. In view of the indicated subsurface stratigraphy, all the soil profile beneath and eastward of the westerly line of the existing warehouses should be considered Type S_4 , and that westward of the warehouses should be considered Type S_3 . Seismic design loadings for new structures should be considered in accordance with the latest BOCA Building Code.

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V. CONCLUDING COMMENTS

This report has been prepared for specific application to the preliminary planning of the Port of Rochester Harbor Improvements and Harbor Ferry Terminal development, in accordance with generally accepted geotechnical engineering practices. It presents a general characterization of the subsurface conditions as Haley & Aldrich has inferred them from the cited data and literature. The actual subsurface conditions between and beyond the points of exploration are expected to vary somewhat from those described and depicted in this report.

The characterizations and geotechnical engineering considerations presented in this report are based, in part, upon the data obtained from the referenced subsurface explorations. The historic construction and uses of the site, together with the geotechnical information presented herein, should be carefully considered in establishing the need for additional exploration, testing, and evaluation to support the design and construction of the anticipated structures and site improvements.

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REFERENCES

1. Herman L. Fairchild (1918), The Rochester Canyon and the Genesee River Base-Levels, Proceedings of the Rochester Academy of Science, October, 1918.

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TABLE I - CONDITIONS ENCOUNTERED IN RECENT (2000) SUBSURFACE INVESTIGATIONS

EXPLORATION IDENTIFICATION	NORTHING	EASTING	SURFACE ELEVATION (ft)	COMPLETION DEPTH (ft)	FILL		ALLUVIUM		LACUSTRINE		GLACIAL TILL		BEDROCK/REFUSAL		COMMENTS
					DEPTH (ft)	ELEV. (ft)	DEPTH (ft)	ELEV. (ft)	DEPTH (ft)	ELEV. (ft)	DEPTH (ft)	ELEV. (ft)	DEPTH (ft)	ELEV. (ft)	
HA-101	1188198	1408785	251.8	115.0	0.00	251.80	8.00	242.80	8.00	243.80	113.00	138.80	113.00	138.80	Weathered rock. Auger refusal @ 115 ft.
HA-102	1188629	1407974	253.5	60.5	0.00	253.50	6.00	247.50	6.00	247.50	15.00	201.00	50.50	201.00	Quarction Formation
HA-103	1187248	1407997	253.86	14	0.00	253.86	18.00	235.86	18.00	235.86	not encountered	below 183	-	-	Weathered rock. Auger refusal @ 115 ft.
HA-104	1187587	1408289	254.25	31	0.00	254.25	19.00	235.25	19.00	235.25	not encountered	below 204	-	-	Weathered rock. Auger refusal @ 210 ft.
HA-105	1187889	1408499	253.96	32	0.00	253.96	15.00	238.96	15.00	238.96	not encountered	below 207	-	-	Quarction Formation
HA-106	1187857	1408244	250.79	41	0.00	250.79	4.00	246.79	4.00	246.79	23.00	243.08	49.00	217.08	Quarction Formation
HA-107	1187375	1407557	266.08	54.5	0.50	265.58	13.00	253.08	13.00	253.08	-	-	-	-	Quarction Formation
HA-108	DELETED	Exploration not made	-	-	-	-	-	-	-	-	-	-	-	-	-
HA-109	1186330	1407602	251.78	27.8	0.00	251.78	2.50	249.28	2.50	249.28	25.00	226.78	25.00	226.78	Weathered rock. Auger refusal @ 27.9 ft.
HA-110	1185999	1407418	252.78	20	0.00	252.78	6.00	246.78	6.00	246.78	20.00	232.78	19.00	232.78	Weathered rock. Auger refusal @ 210 ft.
HA-111	1188376	1408397	251.83	63.5	1.00	250.83	10.00	241.83	10.00	241.83	42.00	209.83	42.00	209.83	Quarction Formation
HA-112	1186945	1407594	250.89	41	0.00	250.89	1.00	249.89	1.00	249.89	19.00	241.89	19.00	241.89	Quarction Formation
HA-113	1188099	1407751	270.8	27	0.00	270.80	20.00	250.80	20.00	250.80	-	-	-	-	Quarction Formation
HA-114	1187858	1407798	261.90	10.3	0.00	261.90	20.00	241.90	20.00	241.90	-	-	-	-	Quarction Formation
HA-114a/1	1187652	1407788	261.92	25	0.00	261.92	14.00	247.92	14.00	247.92	19.00	242.92	19.00	242.92	not encountered
HA-115	1187766	1408031	253.68	26	0.00	253.68	19.00	234.68	19.00	234.68	-	-	-	-	not encountered
HA-116	1187987	1408186	252.44	26	0.40	252.04	8.00	244.44	8.00	244.44	-	-	-	-	not encountered
HA-117/1	1188223	1408074	253.7	27	0.00	253.70	10.00	243.70	10.00	243.70	-	-	-	-	not encountered
HA-118	1188506	1408063	252.78	26	0.00	252.78	2.00	250.78	2.00	250.78	20.00	232.78	20.00	232.78	not encountered
HA-119	1188218	1408530	250.52	51	0.00	250.52	2.00	248.52	2.00	248.52	-	-	-	-	not encountered
HA-120	1187830	1408439	254.31	52	0.00	254.31	6.00	248.31	6.00	248.31	-	-	-	-	not encountered
HA-121	1187786	1407931	276	71	0.00	276.00	4.00	272.00	4.00	272.00	30.00	246.00	30.00	246.00	Red sandstone
HA-122	1187468	1407931	252.8	42	0.00	252.80	14.30	238.50	14.30	238.50	36.00	216.80	36.00	216.80	Quarction Formation
HA-123	1187396	1408120	253.64	116	0.00	253.64	14.00	239.64	14.00	239.64	114.00	139.64	114.00	139.64	Quarction Formation
HA-124	1188260	1407172	267.92	10	0.00	267.92	8.00	257.92	8.00	257.92	-	-	-	-	not encountered
HA-125	1186070	1407222	255.29	10	0.00	255.29	8.00	247.29	8.00	247.29	-	-	-	-	not encountered
BCI-TIP #1			254.00	9.5	0.00	254.00	not encountered	below 245	-	-	-	-	-	-	bottomed on top of pile cap and water @ 9.5 ft.
BCI-TIP #2			252	7	0	252	not encountered	below 245	-	-	-	-	-	-	the-rod&anchor @ 7 ft. water @ 6.75 ft.
BCI-TIP #3a			254	9.5	0	254	not encountered	below 245	-	-	-	-	-	-	the-rod&bottom of anchor @ 10 ft. water @ 9.5 ft.
BCI-TIP #3b			254	10	0	254	not encountered	below 244	-	-	-	-	-	-	top of Sheepleigh deadmen @ 4 ft.
BCI-TIP #4			254	4	0	254	not encountered	below 250	-	-	-	-	-	-	
LBA-TIP #1			251	6	0	251	not encountered	below 246	-	-	-	-	-	-	water infiltration @ 4.5 ft.
LBA-TIP #2			252	8	0	252	not encountered	below 246	-	-	-	-	-	-	perched GW @ 3 ft.
LBA-TIP #3			253	6.3	0	253	3.2	249.8	3.2	249.8	not encountered	below 245	-	-	running sand GW @ 6 ft.
LBA-TIP #4			255	10.3	0	255	1	254	1	254	not encountered	below 247	-	-	standing water @ 5 ft.
LBA-TIP #5			253	4.5	0	253	not encountered	below 249	-	-	-	-	-	-	some sandstone @ bottom. No water mixed.
LBA-TIP #6			252	7	0	252	not encountered	below 246	-	-	-	-	-	-	no water noted.
LBA-TIP #7			252	6.5	0	252	not encountered	below 245	-	-	-	-	-	-	perched GW @ 3 ft.
LBA-TIP #8			260	12	0	260	not encountered	below 248	-	-	-	-	-	-	standing water @ 10.5 ft.
LBA-TIP #9			258	12.5	0	258	5.5	252.5	5.5	252.5	not encountered	below 245.5	-	-	standing water
LBA-TIP #10			269	13	0	269	9.8	259.2	9.8	259.2	not encountered	below 256	-	-	no standing water
LBA-TIP #11			262	5	0	262	not encountered	below 247	-	-	-	-	-	-	bottomed on concrete slab @ 5 ft.
LBA-TIP #12			259	9.5	0	259	not encountered	below 250	-	-	-	-	-	-	bottomed on concrete slab @ 9.5 ft.
LBA-TIP #13			253	7	0	253	not encountered	below 246	-	-	-	-	-	-	standing water @ 7 ft.
LBA-TIP #14			254	9.2	0	254	7.5	251.5	7.5	251.5	not encountered	below 247	-	-	water @ bottom. 9 ft.
LBA-TIP #15			254	11	0	254	1	264	1	264	not encountered	below 254	-	-	water not noted
LBA-TIP #16			254	6.8	0	254	not encountered	below 248	-	-	-	-	-	-	standing water @ bottom. 6.8 ft.
LBA-TIP #17			253	5	0	253	4.5	251.5	4.5	251.5	not encountered	below 246	-	-	water not noted
LBA-TIP #18			256	10	0	256	not encountered	below 245	-	-	-	-	-	-	standing water @ bottom. 5 ft.
LBA-TIP #19			251	6.3	0	251	not encountered	below 247	-	-	-	-	-	-	water not noted
LBA-TIP #20			251	6	0	251	not encountered	below 247	-	-	-	-	-	-	standing water @ bottom. 6 ft.
LBA-TIP #21			251	6	0	251	not encountered	below 247	-	-	-	-	-	-	
LBA-TIP #22			251	6	0	251	not encountered	below 247	-	-	-	-	-	-	

NOTES: 1. ALL ELEVATIONS HAVE BEEN CONVERTED TO CITY OF ROCHESTER DATUM.

TABLE II - CONDITIONS ENCOUNTERED IN EARLIER SUBSURFACE INVESTIGATIONS

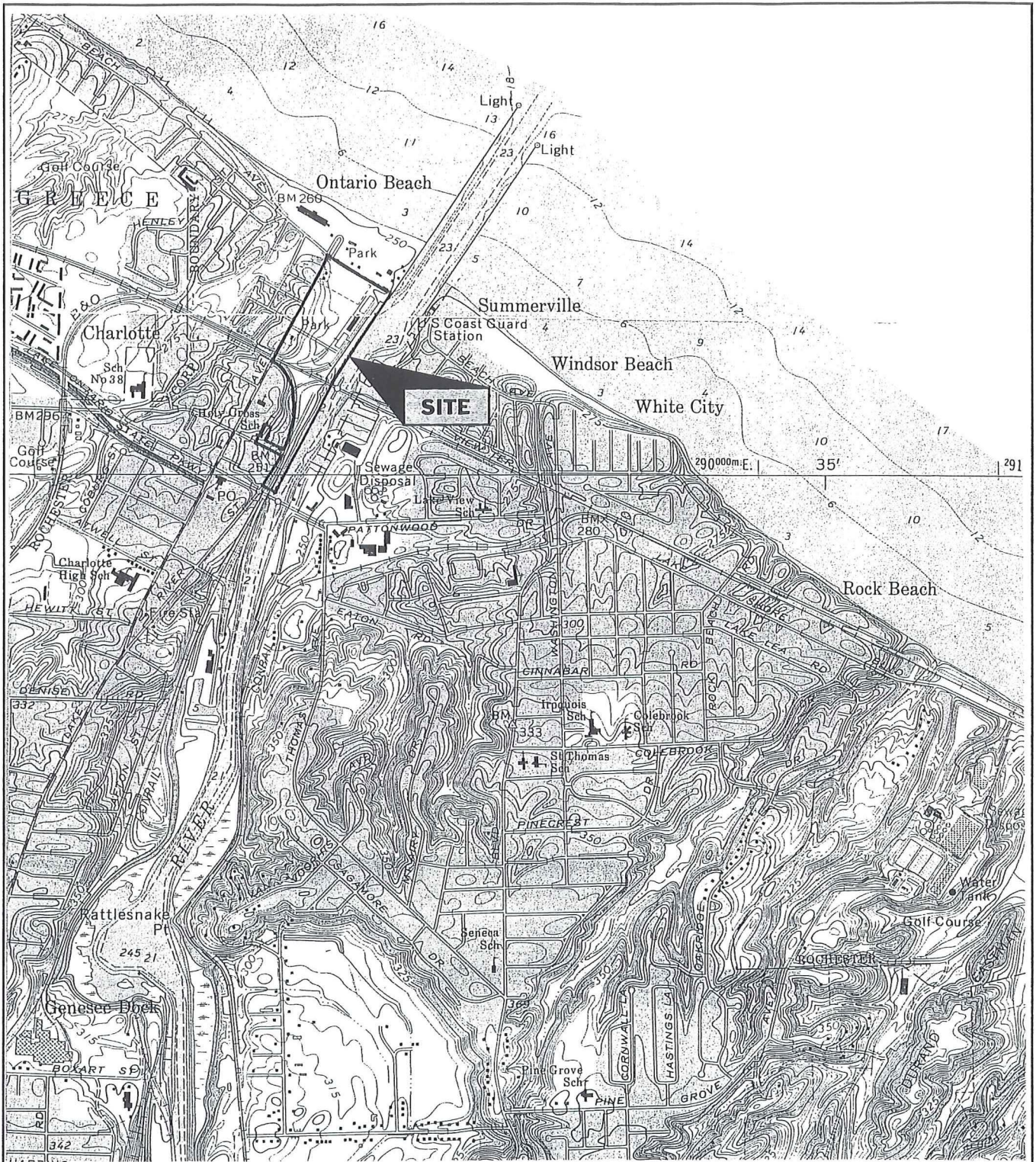
INVESTIGATION TITLE	EXPLORATION IDENTIFICATION	NORTHING	EASTING	SURFACE ELEVATION (ft)	FILL		ALLUVIUM		LACUSTRINE		GLACIAL TILL		BEDROCK/REFUSAL		ROCK DESCRIPTION	
					DEPTH (ft)	ELEV. (ft)	DEPTH (ft)	ELEV. (ft)	DEPTH (ft)	ELEV. (ft)	DEPTH (ft)	ELEV. (ft)	DEPTH (ft)	ELEV. (ft)		DEPTH (ft)
Genesee River Crossing H&A #70037 December-89	B-1	1186769	1407897.1	223.78	-	-	0.00	223.78	-	-	-	-	-	-	-	10.2 ft - sandstone/siltstone
	B-2	1186776	1408024.1	227.48	-	-	0.00	227.48	-	-	-	-	-	-	-	10.2 ft - sandstone/siltstone
	B-3	1186755	1408121.1	224.78	-	-	0.00	224.78	-	-	-	-	-	-	-	10.2 ft - sandstone/siltstone
	B-4	1186736	1408249.1	240.28	-	-	0.00	240.28	-	-	-	-	-	-	-	10.2 ft - sandstone/siltstone
	B-5	1186741	1408381.1	250.58	-	-	0.00	246.58	-	-	-	-	-	-	-	10.2 ft - sandstone/siltstone
	B-6	1186746	1407831.1	251.58	0.00	251.58	4.00	244.08	0.00	246.58	-	-	-	-	-	10.2 ft - sandstone/siltstone
	B-7	1186728	1408435.1	250.58	0.00	250.58	7.50	240.58	10.00	240.58	-	-	-	-	-	10.2 ft - sandstone/siltstone
Sutton Street Bridge NYSDOT December-97	DN-B-3	1185201.1	1407176.2	216.77	-	-	0.00	216.77	-	-	9.20	207.57	-	-	-	12.3 ft - sandstone
	DN-B-4	1185223.4	1407136.1	216.22	-	-	0.00	216.22	-	-	17.50	198.72	-	-	-	10.1 ft - shale/siltstone
	DN-B-5	1185156.4	1407094.9	225.60	-	-	0.00	225.60	-	-	26.30	199.30	-	-	-	10.1 ft - shale/siltstone
	DN-B-51	1185255.8	1407355.8	219.10	-	-	0.00	219.10	-	-	96.90	122.20	-	-	-	9 ft - shale
	DN-B-52	1185220.6	1407341.3	220.91	-	-	0.00	220.91	-	-	100.60	120.31	-	-	-	10.1 ft - shale/siltstone
	DN-B-53	1185182.6	1407326.2	221.30	-	-	0.00	221.30	-	-	100.70	120.60	-	-	-	10.1 ft - shale/siltstone
	DN-B-54	1185110.9	1407304.3	222.88	-	-	0.00	222.88	-	-	100.60	122.38	-	-	-	10.1 ft - shale
	DN-B-55	1185060.8	1407290.5	226.88	-	-	0.00	226.88	-	-	100.60	126.28	-	-	-	10.2 ft - sandstone/siltstone
	DN-B-9	1184953.1	1407594.1	251.67	-	-	0.00	228.79	-	-	-	-	-	-	-	10.2 ft - sandstone/siltstone
	DN-B-1	1185336.5	1408262.2	289.79	0.00	289.79	4.00	285.79	0.00	285.79	34.00	255.79	44.00	245.79	48.40	241.39
DN-B-10	1185222.6	1407944.9	256.98	0.00	256.98	20.00	236.98	0.00	250.89	25.00	231.89	-	-	27.10	229.88	10.3 ft - siltstone
DN-B-11	1184980.6	1407902.7	256.89	0.00	256.89	6.00	250.89	0.00	250.89	25.00	229.89	-	-	27.50	229.39	10.2 ft - siltstone
DN-B-12	1185178.1	1408119.1	254.69	0.00	254.69	10.00	244.69	0.00	244.69	25.00	228.69	-	-	26.10	228.39	10.3 ft - siltstone
DN-B-13	1185203.5	1408309.9	253.18	0.00	253.18	10.00	243.18	0.00	243.18	20.00	223.18	-	-	22.30	230.88	9.9 ft - siltstone
DN-B-14	1185139.5	1408467.2	251.97	0.00	251.97	6.00	245.97	0.00	245.97	25.00	226.97	-	-	25.00	226.97	9.9 ft - siltstone
DN-B-201	1185288.3	1408962.2	289.76	0.00	289.76	1.00	287.08	0.00	287.08	37.00	232.76	-	-	31.00	226.97	8 ft - unidentified bedrock
DN-B-202	1185357.7	1408987.7	288.08	0.00	288.08	1.00	287.08	0.00	287.08	37.00	232.76	-	-	31.00	226.97	8 ft - unidentified bedrock
DN-B-203	1185265.7	1408948.9	291.08	0.00	291.08	1.00	287.08	0.00	287.08	44.00	244.08	-	-	37.00	228.08	10 ft - unidentified bedrock
DN-B-305	1184829.4	1407785.8	253.05	0.00	253.05	16.50	236.55	0.00	236.55	44.00	244.08	-	-	50.00	245.08	4.5 ft - unidentified bedrock
DN-B-306	1184857.5	1407785.9	253.97	0.00	253.97	9.50	244.47	0.00	244.47	25.00	228.05	-	-	25.00	228.05	5 ft - unidentified bedrock
DN-B-307	1184947.3	1407798.5	251.77	0.00	251.77	33.50	218.27	0.00	218.27	40.00	211.77	-	-	25.00	211.77	4 ft - unidentified bedrock
DN-B-308	1184899.9	1407788.5	251.77	0.00	251.77	48.50	203.27	0.00	203.27	55.00	196.77	-	-	55.00	196.77	5 ft - unidentified bedrock
DN-B-309	1184958.8	1407868.8	251.60	0.00	251.60	25.00	226.60	0.00	226.60	100.50	151.10	-	-	100.50	151.10	5 ft - unidentified bedrock
DN-B-310	1184951.4	1407862.5	251.51	0.00	251.51	8.00	243.51	0.00	243.51	8.00	243.51	-	-	8.00	243.51	5 ft - unidentified bedrock
DN-B-311	1184968.8	1407897.6	251.44	0.00	251.44	15.00	236.44	0.00	236.44	15.00	243.37	-	-	15.00	243.37	5 ft - unidentified bedrock
DN-B-312	1185018.8	1407839.2	251.37	0.00	251.37	10.00	242.49	0.00	242.49	107.80	144.69	-	-	107.80	144.69	10.9 ft - unidentified bedrock
DN-B-6	1185024.1	1407567.5	250.49	0.00	250.49	8.00	242.49	0.00	242.49	107.80	144.69	-	-	108.10	143.37	10.9 ft - unidentified bedrock
DN-B-7	1185032.6	1407446.6	251.47	0.00	251.47	6.00	245.47	0.00	245.47	107.80	144.69	-	-	108.10	143.37	10.9 ft - unidentified bedrock
DN-B-8	1185094.7	1407860.6	250.78	0.00	250.78	50.60	200.18	-	200.18	99.60	151.18	-	-	99.60	151.18	10.4 ft - siltstone/shale
Dredge Probes Army Corp June-60	215-734R	1185671.4	1407401.6	1407401.6	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	216-134R	1185763.5	1407441.1	1407441.1	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	216-334R	1185594.2	1407338.6	1407338.6	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	216-534R	1185663.5	1407462.3	1407462.3	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	216-534R	1186028.9	1407572.2	1407572.2	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	216-534R	1186130.0	1407596.8	1407596.8	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	216-534R	1185236.8	1407198.4	1407198.4	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	217-134R	1185508.0	1407288.6	1407288.6	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	217-134R	1185943.2	1407519.3	1407519.3	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	218-33R	1185678.2	1407989.9	1407989.9	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	218-43R	1185767.1	1407434.1	1407434.1	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	218-83R	1186036.9	1407553.9	1407553.9	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	218-93R	1185602.2	1407319.9	1407319.9	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	219-03R	1185888.1	1407454.1	1407454.1	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	219-13R	1185947.9	1407510.8	1407510.8	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	219-83R	1185952.4	1407502.5	1407502.5	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	220-534R	1185970.6	1407449.5	1407449.5	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	220-534R	1185681.1	1407383.8	1407383.8	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	221-83R	1185597.7	1407331.5	1407331.5	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
	222-43R	1185632.2	1407563.7	1407563.7	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL
222-43R	1185510.6	1407280.5	1407280.5	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL	
224-23R	1185687.5	1407373.1	1407373.1	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL	
226-13R	1185776.6	1407416.2	1407416.2	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL	
226-13R	1185116.1	1407288.7	1407288.7	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL	
226-83R	1185782.9	1407404.9	1407404.9	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL	
227-63R	1185691.3	1407365.7	1407365.7	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL	
228-03R	1186039.4	1407547.4	1407547.4	-	-	-	-	-	-	-	-	-	-	-	NO REFUSAL	

TABLE II - CONDITIONS ENCOUNTERED IN EARLIER SUBSURFACE INVESTIGATIONS

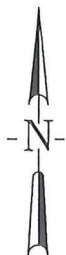
Date: January-10
 Created By: BJAH
 Checked By: SEW

INVESTIGATION TITLE	EXPLORATION IDENTIFICATION	NORTHING	EASTING	SURFACE ELEVATION (ft)	FILL		ALLUVIUM		LACUSTRINE		GLACIAL TILL		BEDROCK/REFUSAL		ROCK DESCRIPTION	
					DEPTH (ft)	TOP ELEV. (ft)	DEPTH (ft)	TOP ELEV. (ft)	DEPTH (ft)	TOP ELEV. (ft)	DEPTH (ft)	TOP ELEV. (ft)	DEPTH (ft)	TOP ELEV. (ft)		DEPTH (ft)
Lake Avenue Improvement Project Vardenhurst March-99	LA-B-1	118975.7	1408991.1	249.23	0.00	249.23	7.50	241.73	7.50	241.73	31.00	218.23	31.00	218.23	>4.5	
	LA-B-2	118979.4	1408564.7	285	0.00	285	>8.1	277	3.50	280	31.00	218.23	31.00	218.23		
	LA-B-3	1186329.1	1406742.5	283	0.00	283	3.50	280	5.00	277	>8.1	218.23	31.00	218.23		
	LA-B-4	1188193.8	1407686	273	0.00	273	5.00	268	5.00	268	>8.2	218.23	31.00	218.23		
	LA-B-5	1188776.2	1407962.7	253	0.00	253	>8.0	246		246						
	LA-B-6	1189868.2	1407675.7	254	0.00	254	>8.1	246	3.30	246	>8.3					
	LA-B-7	1187249.8	1407386.3	280	0.00	280	3.30	277		277						
	LA-B-8	1187249.8	1407158.7	288	0.00	288	>8.2	280		280						
	LA-A-1	1187047.5	1407104.6	282	0.00	282	>3.2	279		279						
	LA-A-2	1187490.9	1407328.6	287	0.00	287	3.50	284		284	>4.1					
LA-A-3	1187842.2	1407504.5	277	0.00	277	1.10	276		276	>4.1						
LA-A-4	1188559.1	1407869.1	259	0.00	259	1.20	258		258	>4.1						
LA-A-5	1188885.2	1407786	253	0.00	253	>3.0	250		250	>4.2						
LA-A-6	1188388.1	140733.9	267	0.00	267	>3.0	264		264							
LA-A-7	1188036.7	1407566.8	276	0.00	276	1.00	275		275	>3.0						
LA-A-8	1188985.8	1408974.2	282	0.00	282	2.10	280		280	>3.1						
LA-A-9	1186531.1	1406798.6	282	0.00	282	1.00	281		281	>3.1						
LA-A-10	1186175.1	1406625.5	284	0.00	284	>3.0	281		281	>3.0						

NOTES: 1. ALL ELEVATIONS HAVE BEEN CONVERTED TO CITY OF ROCHESTER DATUM.
 2. PROBE REFUSAL IS NOT A GUARANTEE OF BEDROCK DEPTH.
 3. NORTHINGS AND EASTINGS ARE BASED ON '83 GRID



70819-000



QUADRANGLE LOCATION: ROCHESTER EAST, N.Y.

HALCYON & ALDRICH

ENGINEERING
PROFESSIONAL
ENVIRONMENTAL
SOLUTIONS

PORT OF ROCHESTER IMPROVEMENTS
AND HARBOR FERRY TERMINAL
ROCHESTER, NY

GEOTECHNICAL CHARACTERIZATION

PROJECT LOCUS

SCALE: AS SHOWN

MARCH 2000

FIGURE 1

See Electronic Copy for Plans

Draft

APPENDIX A

Records of Recent (2000) Subsurface Explorations

- A-1 Logs of Test Pits by Bourne Consulting Engineers**
- A-2 Logs of Test Pits by LaBella Associates**
- A-3 Logs of Test Borings by Haley & Aldrich**
- A-4 Observation Well Installation Reports by Haley & Aldrich**

H&A OF NEW YORK, ROCHESTER, NEW YORK Consulting Geotechnical Engineers, Geologists and Hydrogeologists			TEST PIT REPORT		TEST PIT NO. <u>TP#1</u>
PROJECT: <u>Part of Rochester - Charlotte Beach, New York</u>					LOCATION: <u>Adjacent to Boat launch</u>
LOCATION: <u>Old General Cargo Terminal</u>					ELEVATION: <u>Not Surveyed</u>
CLIENT: <u>Labella Associates (City of Rochester)</u>					EXPLORATION DATE: <u>11 Jan 2000</u>
CONTRACTOR: <u>Hickory Hill Construction</u>					H&A REP.: <u>R. Dedrick</u>
EQUIPMENT USED: <u>John Deere 410D</u>					
SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS	REMARKS
			<u>0.5ft</u>	<u>Asphalt</u>	
				<u>Grey medium to coarse sandy GRAVEL. Little silt. Dry</u>	① Cobbles composition unknown. Iridescent blue & sulfur smell. Possible Foundry byproduct.
<u>2</u>			<u>2.0ft</u>	<u>- Crushed Stone -</u>	
	<u>obtained by Labella Associates</u>			<u>Brown red fine to coarse SAND. Some cobbles. Some coarse gravel. Moist</u>	② More dense composition of cobbles located adjacent to river-wall
<u>4</u>					③ Tie-rods located adjacent to triangular concrete forms.
<u>6</u>					
<u>8</u>					
<u>10</u>			<u>9.5ft</u>	<u>Top of Pile cap - Concrete structure</u> <u>BOE = 9.5ft</u>	
<u>12</u>				<u>* exploration ended due to influx of water</u>	
WATER LEVEL			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME*	DEPTH FT	LENGTH	WIDTH	DEPTH:
<u>11 Jan 00</u>	<u>30min</u>	<u>9.5ft</u>	<u>25</u> feet	<u>20</u> feet	<u>9.5 ft</u>
			BOULDERS		JAR SAMPLES: -
			8" to 18" DIAMETER: No. <u>50+</u> = Vol.	cu ft	BAG SAMPLES: -
			Over 18" DIAMETER: No. <u>10+</u> = Vol.	cu ft	WATER LEVEL: <u>Not Present</u>
* Hrs after completed					TEST PIT NO. <u>TP # 1</u>

H&A OF NEW YORK, ROCHESTER, NEW YORK Consulting Geotechnical Engineers, Geologists and Hydrogeologists	TEST PIT REPORT	TEST PIT NO. TP# 2 FILE NO. 70819-000
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PROJECT: LOCATION: CLIENT: CONTRACTOR: EQUIPMENT USED:	LOCATION: Between Buildings ELEVATION: Not Surveyed EXPLORATION DATE: 12 Jan 2000 H&A REP.: R. Dedrick
--	---

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS	REMARKS
		0.3ft		Asphalt	① Located between building above deadman anchor ② 30 ft swing tie from corner of both buildings to tie rod ③ Completed in 2 sections. Similar cross sections. However, 2 pit (closest to river) encountered railroad ties & steel @ ~ 0.5ft
		1.0ft		Crushed Stone	
-2	Obtained by Labella Associates			Brown silty fine to coarse SAND. Little coarse gravel. Trace cobbles Trace clay. Moist. Pockets of grey/green discolored soil. Pocket of crushed brick.	
-4					
-6					
-8		7.0ft		Located Tie-line & Anchor BOE = 7.0ft	
-10					
-12					

WATER LEVEL			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME*	DEPTH FT	LENGTH	WIDTH	
12 Jan 00	15 min	6.75ft	50 feet	10 feet	DEPTH: 7.0ft JAR SAMPLES: - BAG SAMPLES: - WATER LEVEL: 6.75ft
			BOULDERS		
			8" to 18" DIAMETER: No. 15+ = Vol.	cu ft	
			Over 18" DIAMETER: No. 0 = Vol.	cu ft	
* Hrs after completed					TEST PIT NO. TP# 2

H&A OF NEW YORK, ROCHESTER, NEW YORK Consulting Geotechnical Engineers, Geologists and Hydrogeologists	TEST PIT REPORT	TEST PIT NO. <u>TP# 3a</u> FILE NO.
--	-----------------	--

PROJECT: LOCATION: CLIENT: CONTRACTOR: EQUIPMENT USED:	LOCATION: <u>South of Paddle boat House</u> ELEVATION: <u>Not Surveyed</u> EXPLORATION DATE: <u>12 Jan 2000</u> H&A REP.: <u>R. Dedrick</u>
--	--

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS	REMARKS
			0.3ft	----- <u>Asphalt</u> -----	① Encountered railroad tracks approximately 0.5ft below ground surface 7.0ft inland of river wall. ② 2nd set of railroad track similar to first set located approximately 20ft in from river wall
			1.0ft	----- <u>Crushed Stone</u> -----	
- 2	Obtained by Labella Associates			Light brown fine to coarse <u>SAND</u> . Some silt. Moist.	
- 4					
- 6					
- 8					
- 10			9.5ft	<u>Located top of pile cap.</u> <u>BOE = 9.5</u>	
- 12				* exploration ended due to influx of water	

WATER LEVEL			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME*	DEPTH FT	LENGTH	WIDTH	DEPTH:
12 Jan 00	30 min	9.5ft	20 feet	15 feet	9.5ft
			BOULDERS		JAR SAMPLES: -
			8" to 18" DIAMETER: No. \emptyset = Vol.	cu ft	BAG SAMPLES: -
			Over 18" DIAMETER: No. \emptyset = Vol.	cu ft	WATER LEVEL: 9.5ft
* Hrs after completed					TEST PIT NO. <u>TP# 3a</u>

H&A OF NEW YORK, ROCHESTER, NEW YORK Consulting Geotechnical Engineers, Geologists and Hydrogeologists	TEST PIT REPORT	TEST PIT NO. TP#3b FILE NO.
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PROJECT: LOCATION: CLIENT: CONTRACTOR: EQUIPMENT USED:	LOCATION: Southwest of Paddleboat House ELEVATION: Not surveyed EXPLORATION DATE: 13 Jan 2000 H&A REP.: R. Dedrick
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SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS	REMARKS
			0.2ft	Concrete Foundation	① Strata change @ 5.0ft is even with top of Sheetpile on deadman anchor
			0.7ft	Crushed Stone / Asphalt Mixture	
2	No Samples Taken			Brown fine to coarse SAND. Some silt. Trace coarse gravel. Moist.	
4					
6			5.0ft	Dark brown grey silty fine to coarse SAND. Some coarse gravel. Little cobbles. Concrete debris.	
8					
10			10.0ft	Located Tie-rope / Bottom of anchor BOE = 10.0ft	
12					

WATER LEVEL			APPROXIMATE PIT DIMENSIONS AT SURFACE		SUMMARY
DATE	TIME*	DEPTH FT	LENGTH	WIDTH	
12 Jan 00	30min	9.5 ft	20 feet	15 feet	DEPTH: 10.0 ft JAR SAMPLES: - BAG SAMPLES: - WATER LEVEL: 9.5 ft TEST PIT NO. TP#3b
			BOULDERS		
			8" to 18" DIAMETER: No. 204 = Vol.	cu ft	
* Hrs after completed			Over 18" DIAMETER: No. \emptyset = Vol.	cu ft	

H&A OF NEW YORK, ROCHESTER, NEW YORK Consulting Geotechnical Engineers, Geologists and Hydrogeologists	TEST PIT REPORT	TEST PIT NO. TP#4 FILE NO. 70819-000
PROJECT: LOCATION: CLIENT: CONTRACTOR: EQUIPMENT USED:	LOCATION: North of paddleboat house ELEVATION: Not Surveyed EXPLORATION DATE: 13 Jan 99 H&A REP.: R. Dedrick	

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS	REMARKS
2			0.8ft	Organic	① Located north of paddleboat house at intersection of 2-deadmen anchors.
4				Brown silty fine to coarse SAND. Little gravel .. Little cobbles. Brick debris.	
6			4.0	Top of Sheet Pile	
8				< Left site to return to office >	
10					
12					

WATER LEVEL			APPROXIMATE PIT DIMENSIONS AT SURFACE				SUMMARY
DATE	TIME*	DEPTH FT	LENGTH	feet	WIDTH	feet	DEPTH: Left before Completion JAR SAMPLES: - BAG SAMPLES: - WATER LEVEL: -
			BOULDERS				
			8" to 18" DIAMETER:	No.	= Vol.	cu ft	
			Over 18" DIAMETER:	No.	= Vol.	cu ft	
* Hrs after completed							TEST PIT NO. TP#4

TEST PIT REPORT

BELLA ASSOCIATES, P.C. 300 STATE STREET			TEST PIT # / PROJECT # 99150 DATE: 08	
PROJECT: Part LOCATION: Drilling lot @ P.L. Twin TABLE CLIENT: CONTRACTOR: Hickory Hills EQUIPMENT: BACK-HOE			ELEVATION: LABELLA REP: DEP	
SCALE	SAMPLE	SAMPLE	DESCRIPTION OF MATERIALS	REMARKS
IN	NUMBER	DEPTH		
FEET		RANGE		
1			- Checkup RED/BLACK CINDERS MISC. FILL	NP 0
2			MED/COURSE BROWN SAND	0
3			RAILROAD TIES ← RAILROAD TIES	0
4			↓ WATER INFILTRATION (DESIG?/ACTUAL?)	0
5			↓ RUNNING SAND/GW @ 6	0
6				
7				
8				
9				
10				
11				
12				
13				
WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE	
DATE	TIME*	DEPTH	WIDTH X LENGTH =	
Hrs. after completion			TEST PIT #1	

TEST PIT REPORT

BELLA ASSOCIATES, P.C. 300 STATE STREET				TEST PIT # 2 PROJECT # 99150 DATE: 2/23/00	
SUBJECT: LOCATION: EVENT: CONTRACTOR:				ELEVATION: LABELLA REP: DE	
EQUIPMENT:		BACK-HOE			
SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS		REMARKS
1			Blacktop Red silt/sand with gravel	16 000 ↓	0
2			Gray med/coarse sand		0
3			↓ Med Gravel		0
4			↓ Requies? / ACTUAL GW		0
5			↓ Standing GW - Term		
6			↓		
7					
8					
9					
10					
11					
12					
13					
WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE WIDTH X LENGTH =		
DATE	TIME*	DEPTH			
* Hrs. after completion			TEST PIT #1		

TEST PIT REPORT

BELLA ASSOCIATES, P.C. 300 STATE STREET				TEST PIT # 3 PROJECT # 99150 DATE: 2/28/00	
PROJECT: LOCATION: CLIENT: CONTRACTOR:				ELEVATION: LABELLA REP:	
EQUIPMENT: BACK-HOE					
SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS	
1			Blacktop 25% silt/sand w/ Gravel Brown/gray sand	0	
2			↓ some gravel ↓ running sand / GW	0	
3				0	
4				0	
5				0	
6				0	
7				0	
8				0	
9				0	
10				0	
11				0	
12				0	
13				0	
WATER LEVEL				APPROXIMATE TEST PIT DIMENSIONS AT SURFACE WIDTH X LENGTH =	
DATE	TIME*	DEPTH			
			TEST PIT #1		

Hrs. after completion

TEST PIT REPORT

ABELLA ASSOCIATES, P.C.			TEST PIT # 4	
300 STATE STREET			PROJECT # 99150	
			DATE: 2/28/00	
PROJECT:			ELEVATION:	
LOCATION:			ABELLA REP: DEF	
CLIENT:				
CONTRACTOR:				
EQUIPMENT: BACK-HOE				
SCALE	SAMPLE	SAMPLE	DESCRIPTION OF MATERIALS	REMARKS
IN	NUMBER	DEPTH		
FEET		RANGE		
1			Blacktop	
2			White conc? misc. fill	0
3			silt/sand	
4			Brown red sand	0
5			↓ spray H ₂ O -	0
6			↓	0
7				
8				
9				
10				
11				
12				
13				
WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE	
DATE	TIME*	DEPTH	WIDTH X LENGTH =	
* Hrs. after completion			TEST PIT #1	

SOME BLUE SLAG
(S⁺ FOR O⁺)

/// LAYER OF SLAG (dense) ///

NO
O⁺

NO
O⁺

TEST PIT REPORT

BELLA ASSOCIATES, P.C. 300 STATE STREET	TEST PIT # 3 PROJECT # 99150 DATE 2/28/00
PROJECT: LOCATION: IDENT: CONTRACTOR:	ELEVATION: LABELLA REP:

EQUIPMENT:		BACK-HOE	DESCRIPTION OF MATERIALS	REMARKS
SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE		
1			Grass silt/sand some gravel	0
2			Brown sand	
3			↓	
4			silt sand some clay	0
5			↓	
6			↓	
7			↓	
8			↓	0
9			fine sand some gravel	
10			↓	
11			↓	
12			↓	
13			↓	0

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE	
DATE	TIME*	DEPTH	WIDTH X LENGTH =	
Hrs. after completion			TEST PIT #1-	

TEST PIT REPORT

BELLA ASSOCIATES, P.C.
 300 STATE STREET

TEST PIT # 6
 PROJECT # 99150
 DATE: 2/28/06

PROJECT:
 LOCATION:
 CLIENT:
 CONTRACTOR:

ELEVATION:
 LABELLA REP:

EQUIPMENT: BACK-HOE

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			Grass bed with gravel misc. fill	
2			Blue "sulfur rocks" misc fill	
3				
4			↓ Termin @ 4' DUE TO SLAG	
5				misc. white "slag" (sample)
6				
7				
8				
9				
10				
11				
12				
13				

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE WIDTH X LENGTH =
DATE	TIME	DEPTH	

* Hrs. after completion

TEST PIT #1

TEST PIT REPORT

ABELLA ASSOCIATES, P.C.

300 STATE STREET

TEST PIT # 7

PROJECT # 99150

DATE: 2/29/04

PROJECT:
LOCATION:
CLIENT:
CONTRACTOR:

ELEVATION:

ABELLA REP:

EQUIPMENT: BACK-HOE

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			COARSE misc silt/gravel	
2			Blue slag	golf ball dent ↓
3			misc fill - brick / slag / concrete	
4			↓ - Shell layer sample	
5			↓ H ₂ O	
6			↓	
7			misc. fill	
8				
9				
10				
11				
12				
13				

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE WIDTH X LENGTH =
DATE	TIME*	DEPTH	

* Hrs. after completion

TEST PIT #1

TEST PIT REPORT

ABELLA ASSOCIATES, P.C.

TEST PIT # 8
PROJECT # 99150
DATE: 2/28/00

300 STATE STREET

OBJECT:

ELEVATION:

LOCATION:

ABELLA REP:

AGENT:

CONTRACTOR:

EQUIPMENT:

BACK-HOE

DEPTH IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			60SS misc. fill - slag / brick	sulfur odor
2			black FINE ASH/silt?	
3			slag. misc. fill	
4				
5			GW @	
6				
7				
8				
9				
10				
11				
12				
13				

WATER LEVEL

APPROXIMATE TEST PIT DIMENSIONS AT SURFACE

DATE	TIME*	DEPTH

WIDTH X LENGTH =

Time after completion

TEST PIT #1

TEST PIT REPORT

BELLA ASSOCIATES, P.C.

TEST PIT # 9

PROJECT # 99150

DATE: 2/28/00

300 STATE STREET

PROJECT:

ELEVATION:

LOCATION:

CLIENT:

LABELLA REP:

CONTRACTOR:

EQUIPMENT: BACK-HOE

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			GRAVEL SAND	0
2			↓ sample RED SLAG - Fill / misc. + BLUE SLAG	0
3				
4			ASH (sample)	0
5				
6			STANDING WATER (no stream)	0
7				
8				0
9				0
10				0
11				0
12				0
13				0

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE WIDTH X LENGTH =
DATE	TIME*	DEPTH	

Hrs. after completion

TEST PIT #1

TEST PIT REPORT

BELLA ASSOCIATES, P.C.

300 STATE STREET

TEST PIT # 10

PROJECT # 99158

DATE: 2/28/00

PROJECT:

ELEVATION:

LOCATION:

AGENT:

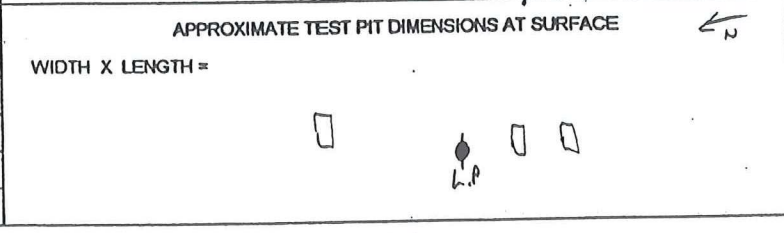
LABELLA REP:

CONTRACTOR:

EQUIPMENT: BACK-HOE

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS				REMARKS
			1	2	3	4	
1			Grass sand/silt	Grass sand/silt	Grass Gravel	Grass silt/fill	
2				slag-red/blue	Conc.	Red silt? fill	
3			Conc. Slab	Large log 3'		Black crushed fill	
4						Brown sand	
5			Conc.			Gray f. sand - firm	
6						Brown sand	
7							
8							
9							
10							
11							
12						(NO STANDING SW)	
13						Hard sand/TILL layer	

WATER LEVEL		
DATE	TIME*	DEPTH



TEST PIT #1

hrs. after completion

TEST PIT REPORT

LABELLA ASSOCIATES, P.C. 300 STATE STREET	TEST PIT # 11 PROJECT # 99150 DATE: 2/28/00
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PROJECT: LOCATION: CLIENT: CONTRACTOR: EQUIPMENT: BACK-HOE	ELEVATION: LABELLA REP:
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SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			Gravel silt/sand Browns (some debris/conc. chips)	no odor
2				c
3				e
4				e
5				D
6				D
7				D
8				D
9				D
10				D
11				C
12				C
13				C

↓

Grey silt (brown) / clay

↓

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE	
DATE	TIME*	DEPTH	WIDTH X LENGTH =	
* Hrs. after completion			TEST PIT #1	

TEST PIT REPORT

BELLA ASSOCIATES, P.C.
 300 STATE STREET

TEST PIT # 12
 PROJECT # 9915
 DATE: 2/28/05

OBJECT:
 LOCATION:
 IDENT:
 CONTRACTOR:

ELEVATION:
 LABELLA REP:

EQUIPMENT: BACK-HOE

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			Grass Brick/rock frag.	
2			Misc. fill	
3			blue/brk slag	
4				
5			brick Brick/conc. PZZZZ	
6				
7				
8				
9				
10				
11				
12				
13				

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE WIDTH X LENGTH =
DATE	TIME*	DEPTH	

* Hrs. after completion

TEST PIT #1

TEST PIT REPORT

ABELLA ASSOCIATES, P.C. 300 STATE STREET	TEST PIT # 13 PROJECT # 99153 DATE: 2/29/00
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PROJECT: LOCATION: CLIENT: CONTRACTOR:	ELEVATION: LABELLA REP:
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SCALE	SAMPLE	SAMPLE	DESCRIPTION OF MATERIALS	REMARKS
IN	NUMBER	DEPTH		
FEET		RANGE		
1			Gravel/sub-base	
2			silt/sand w/ gravel	
3			FIRM/DENSE HARD FINE SAND	
4			↓ Brick/Conc.	
5			Brown sand	
6			↓	
7			↓	
8			Black Cinders	
9			↓	
10			Conc. slab.	
11				
12				
13				

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE	
DATE	TIME*	DEPTH	WIDTH X LENGTH =	

Hrs. after completion	TEST PIT #1
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TEST PIT REPORT

BELLA ASSOCIATES, P.C. 300 STATE STREET	TEST PIT # 14 PROJECT # 99150 DATE: 2/29/03
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PROJECT: LOCATION: CLIENT: CONTRACTOR: EQUIPMENT:	BACK-HOE ELEVATION: LABELLA REP:
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SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			Gravel - sub-base Misc. fill (slag, ^{blue} gravel, stones, brick)	no odor
2			↓	sulfur odor
3				
4				
5				
6				
7				↓
8			STANDING WATER	
9				
10				
11				
12				
13				

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE	
DATE	TIME*	DEPTH	WIDTH X LENGTH =	
* Hrs. after completion			TEST PIT #1	

TEST PIT REPORT

BELLA ASSOCIATES, P.C. 300 STATE STREET			TEST PIT # 15 PROJECT # 99150 DATE: 2/29/00	
PROJECT: LOCATION: CLIENT: CONTRACTOR:			ELEVATION: LABELLA REP:	
EQUIPMENT: BACK-HOE				
SCALE	SAMPLE	SAMPLE	DESCRIPTION OF MATERIALS	REMARKS
IN	NUMBER	DEPTH	GENREL/SUB-BASE silt/sand gravel (gall) CONC. SLAB misc. slag. (white - sample) (IRON - sample) ↓ H ₂ O	
FEET	RANGE	RANGE		
1				
2				
3				
4				
5				
6				
7				
8				
9				
10				
11				
12				
13				
WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE	
DATE	TIME*	DEPTH.	WIDTH X LENGTH =	
* Hrs. after completion			TEST PIT #1	

TEST PIT REPORT

BELLA ASSOCIATES, P.C.
300 STATE STREET

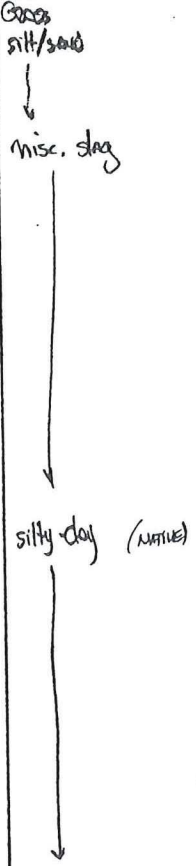
TEST PIT # 16
PROJECT # 99150
DATE: 2/27/00

OBJECT:
LOCATION:
AGENT:
CONTRACTOR:
EQUIPMENT:

ELEVATION:
LABELLA REP:

BACK-HOE

DEPTH IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			Coars silt/sand	
2			↓ misc. slag	
3				
4				
5				
6				
7				
8			↓ silty clay (NATIVE)	
9				
10				
11				
12				
13				



WATER LEVEL		
DATE	TIME*	DEPTH

APPROXIMATE TEST PIT DIMENSIONS AT SURFACE
WIDTH X LENGTH =

* Hrs. after completion

TEST PIT #1

TEST PIT REPORT

BELLA ASSOCIATES, P.C. 300 STATE STREET				TEST PIT # 11 PROJECT # 99150 DATE: 2/29/08	
PROJECT: LOCATION: CLIENT: CONTRACTOR:				ELEVATION: LABELLA REP:	
EQUIPMENT: BACK-HOE					
SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS	
1			Grass topsoil/silt	No 000 ↓	0
2			med brown sand/silt		
3			↓ Gray blue clay (silt)		0
4					
5					
6					0
7					
8					
9					0
10					
11					
12					
13					
WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE		
DATE	TIME	DEPTH	WIDTH X LENGTH =		
Hrs. after completion			TEST PIT #1		

TEST PIT REPORT

BELLA ASSOCIATES, P.C.
 300 STATE STREET

TEST PIT # 18
 PROJECT # 99150
 DATE: 2/29/00

PROJECT:
 LOCATION:
 CLIENT:
 CONTRACTOR:

ELEVATION:
 LABELLA REP:

EQUIPMENT: BACK-HOE

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
---------------	---------------	--------------------	--------------------------	---------

1			Gravel silt/sand - gravel	0
2			Misc. slag white, blue, green	Sulfur odor
3				
4			STAND. WATER	0
5				
6				0
7				
8				
9				
10				
11				
12				
13				

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE WIDTH X LENGTH =
DATE	TIME*	DEPTH	

* Hrs. after completion

TEST PIT #1

TEST PIT REPORT

BELLA ASSOCIATES, P.C. 300 STATE STREET				TEST PIT # 19 PROJECT # 9958 DATE: 2/25/02
PROJECT: LOCATION: CLIENT: CONTRACTOR:				ELEVATION: LABELLA REP:
EQUIPMENT: BACK-HOE				
SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			Grass silt sand	
2			↓	
3				
4			dense slag - white/blue	
5			sand. water - some steel	
6				
7				
8				
9				
10				
11				
12				
13				
WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE	
DATE	TIME*	DEPTH	WIDTH X LENGTH =	
* Hrs. after completion			TEST PIT #1	

TEST PIT REPORT

BELLA ASSOCIATES, P.C.

TEST PIT # 20
PROJECT # 99150
DATE: 2/2/00

300 STATE STREET

OBJECT:
LOCATION:
IDENT:
CONTRACTOR:

ELEVATION:
LABELLA REP:

EQUIPMENT: BACK-HOE

DEPTH IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			GRASS Silt/sand/Topsoil RED COARSE SAND - WASTE FILL	NP 0cm
2				
3				0
4				
5			Brown silt / FINE SAND	
6				0
7				
8				0
9				
10				
11				
12				
13				

NO
SLAG (Rocks)

WATER LEVEL		
DATE	TIME*	DEPTH

APPROXIMATE TEST PIT DIMENSIONS AT SURFACE
WIDTH X LENGTH =

TEST PIT #1

Hrs. after completion

TEST PIT REPORT

BELLA ASSOCIATES, P.C.

300 STATE STREET

TEST PIT # 21
PROJECT # 99150
DATE: 2/29/00

OBJECT:

LOCATION:

AGENT:

CONTRACTOR:

ELEVATION:

LABELLA REP:

EQUIPMENT: BACK-HOE

DEPTH SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			Black top 2" Gravel (over sub-base) Gray med/fine sand	ug over
2			sample → create color	create color
3				
4				
5			TIES	
6			sample SLAB	
7				
8				
9				
10				
11				
12				
13				

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE WIDTH X LENGTH =
DATE	TIME*	DEPTH	

Hrs. after completion

TEST PIT #1

TEST PIT REPORT

BELLA ASSOCIATES, P.C.
 300 STATE STREET

TEST PIT # 22
 PROJECT # 9950
 DATE: 2/29/00

PROJECT:
 LOCATION:
 PERMIT:
 CONTRACTOR:

ELEVATION:
 LABELLA REP:

EQUIPMENT:		BACK-HOE		
SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	DESCRIPTION OF MATERIALS	REMARKS
1			Coarse silt/sand soil	no obs
2			↓ misc. slag layer - blue/white	Subtle obs
3			↓	
4			↓	
5			↓	
6			conc. slag layer - Hoel Row	
			~ ~ ~ ~ ~ STANDING WATER - SCREEN	
7				
8				
9				
10				
11				
12				
13				

WATER LEVEL			APPROXIMATE TEST PIT DIMENSIONS AT SURFACE
DATE	TIME*	DEPTH	
			WIDTH X LENGTH =

* Hrs. after completion

TEST PIT #1

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	23-May-00
DRILLER	L. TODD	DATE FINISHED	23-May-00

Elevation	251.8	ft	Datum	City	Boring Location	See Boring Location Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount	
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head
						Drill Mud
						<input type="checkbox"/> Bentonite
						<input type="checkbox"/> Polymer
						<input checked="" type="checkbox"/> None
						Casing <input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						CONCRETE
		26	S1	1.0		Very dense black brown gray silty fine to coarse SAND, some rock fragments, dry.
		26	10"/12"	2.0		
		9	S2	2.0		Medium dense black brown silty fine to coarse SAND, some rock fragments, moist.
		10				
		18				
		15	12"/24"	4.0		Medium dense black brown yellow, silty fine to coarse SAND, some rock fragments, wet.
		4	S3	4.0		
5		8				
		5	7"/24"	6.0		Same.
		7	S4	6.0		
		17				FILL
		18				
		17	10"/24"	8.0		Very dense black brown silty fine to coarse SAND, some rock fragments. Obstruction at 8.0 ft.
		100/2	S5	8.0	8.2	
10						See Boring HA-101a (HA-101a offset 30' West of Original Location)
15						
20						
25						
30						

Water Level Data						Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	Open End Rod	Overburden (Linear ft)	115
						T	Thin Wall Tube	Rock Cored (Linear ft)	---
						U	Undisturbed Sample	Number of Samples	18S
						S	Split Spoon Sample	BORING NO. HA-101	
						G	Geoprobe		

TEST BORING REPORT

BORING NO.

HA-101a

Page 2 of 5

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	7-Jun-00
DRILLER	L. TODD	DATE FINISHED	7-Jun-00

Elevation	251.8	ft	Datum	City	Boring Location	See Boring Location Plan			
Item	Casing	Sampler	Core Barrel	Rig Make & Model			Drill Mud		
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type		
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Bentonite	
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input type="checkbox"/> Polymer	
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven <input type="checkbox"/> Spun	

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						Augered to 5.0 ft.
5	5	5	S6	5.0		Loose gray brown fine to medium SAND, trace silt, organics, moist.
		3				ALLUVIUM
		5	14"/24"	7.0		Augered to 10.0 ft.
10	4	4	S7	10.0		Same, except wet.
		3				
		2	16"/24"	12.0		
15	1	9	S8	15.0		Medium dense gray brown fine to coarse SAND, some coarse gravel, wet.
		7				
		7	23"/24"	17.0		
20	11	10	S9	20.0		Same.
		11				
		14	20"/24"	22.0		
25	12	3	S10	25.0		Same, except loose.
		4				
		3	20"/24"	27.0		
30						

Water Level Data						Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	T	Overburden (Linear ft)	115
						U <td>S <th>Rock Cored (Linear ft)</th> <td>---</td> </td>	S <th>Rock Cored (Linear ft)</th> <td>---</td>	Rock Cored (Linear ft)	---
						G <td> <th>Number of Samples</th> <td>18S</td> </td>	<th>Number of Samples</th> <td>18S</td>	Number of Samples	18S
								BORING NO.	HA-101a

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
		9	S11	30.0		Loose gray brown silty fine to coarse SAND, trace gravel, wet.
		4				
		4				ALLUVIUM
		3	22"/24"	32.0		
35		2	S12	35.0		Same, except very loose.
		1				
		2				
		2	23"/24"	37.0		
40		1	S13	40.0		Very loose gray brown fine sandy SILT, little clay, organics, wet.
		1				
		2				
		3	22"/24"	42.0		
45		1	S14	45.0		Same, except no organics.
		2				
		2				
		3	18"/24"	47.0		
50		1	S15	50.0		Same.
		2				
		2				
		2	20"/24"	52.0		
55		1	S16	55.0		Same.
		2				
		3				
		5	20"/24"	57.0		
60		1	S17	60.0		Loose gray brown SILT, little clay, trace sand, wet.
		3				
		4				
		4	20"/24"	62.0		
65		1	S18	65.0		Same.
		2				
		2				
		4	23"/24"	67.0		
						(Augered to bedrock)
70						

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
						(Augered to bedrock - No samples recovered)
75						
80						
85						
90						
95						
100						
105						
110						
					FILE NO.	70819-000
					BORING NO.	HA-101a

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
		10	S10	30.0		Very dense gray brown silty fine to coarse SAND, little rock fragments, wet. GLACIAL TILL
		100/3	7"/10"	30.8		
35		19	S11	35.0		Very dense silty fine to coarse SAND, some gravel, trace clay, wet.
		60 100/3	16"/16"	36.3		
40		15	S12	40.0		Same.
		100/4	10"/11"	40.9		
45		20	S13	45.0	45.0	Very dense red brown silty fine to coarse SAND, trace clay, moist. WEATHERED ROCK
		100/1	6"/8"	45.6		
50		100/3	S14	50.0	50.5	Same, with little clay. Began rock coring at 50.5 ft.
				50.3		
55						Competent, red sandstone with interbedded gray sandstone. QUEENSTONE FORMATION
60						Highly fractured 58.8 ft. to 60.5 ft.
						Bottom of Exploration at 60.5 ft.
65						
70						
					FILE NO.	70819-000
					BORING NO.	HA-102

CORE BORING REPORT

BORING NO.

HA-102

Page 1 of 1

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISE	DATE STARTED	30-May-00
DRILLER	L. TODD	DATE FINISHED	30-May-00

Elevation	ft	Datum		Boring Location				Drill Mud			
Item	Casing	Sampler	Core Barrel	Rig Make & Model		Hammer Type		Drill Mud			
Type	HAS	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type		<input type="checkbox"/> Bentonite		
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety		<input type="checkbox"/> Polymer		
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut		<input checked="" type="checkbox"/> None		
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing		<input type="checkbox"/> Driven <input type="checkbox"/> Spun		

Depth (ft)	Drilling Rate (min/ft)	Core No. Depth (ft)	Recovery RQD		Weathering	Stratum Change (ft)	Visual Classification and Remarks
			(in)	(%)			
		50.5					
			3.4/5.0	68			Competent red sandstone with interbedded gray sandstone.
							QUEENSTONE FORMATION
5	Avg. 3-4 minutes per foot						
			3.45/5.0	69			Highly fractured 8.0 ft. to 10.0 ft.
10		60.5					
15							
20							
25							
30							

Water Level Data						Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	T	Overburden (Linear ft)	50.5
								Rock Cored (linear ft)	10
								Samples	14S
								BORING NO. HA-102	

TEST BORING REPORT

BORING NO.
HA-103a
Page 2 of 3

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	31-May-00
DRILLER	L. TODD	DATE FINISHED	1-Jun-00

Elevation	253.86	ft	Datum	City	Boring Location	See Boring Location Plan		
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount			Drill Mud
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type	<input type="checkbox"/> Bentonite
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Polymer
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> None
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0		A				(Offset 18 west of original location)
		U				
		G	See Samples for 0-14 ft. in Boring HA-103			
		E				
		R				
5		11		5.0		Medium dense brown black fine to coarse SAND, little silt, dry.
		14				
		7				FILL
		4		7.0		
		A				
		U				
		G				
		E				
		R				
10		9		10.0		Dense black brown fine to coarse SAND, little silt, slag fragments, wet.
		19				
		22				
		20		12.0		
		7	S8	14.0		Same, except medium dense.
15		11				
		15				
		12	14"/24"	16.0		
		8				
		3	No Recovery	18.0		
		6				
		7	S9	18.0		Medium dense black brown silty fine to coarse SAND, wet.
		8				
		10				ALLUVIUM
		8				
20		6	6"/24"	20.0		
		5	S10	24.0		Loose gray brown fine sand SILT, wet.
25		3				
		3				
		4	2"/24"	26.0		
		2	S11	29.0		Loose gray fine sand SILT, some clay, organics, moist.
30		3				
		3	12"/18"	30.3		

Water Level Data						Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	Open End Rod	Overburden (Linear ft)	7
						T	Thin Wall Tube	Rock Cored (Linear ft)	--
						U	Undisturbed Sample	Number of Samples	19S
						S	Split Spoon Sample	BORING NO. HA-103a	
						G	Geoprobe		

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
35		1	S12	34.0		Loose gray fine sand SILT, some clay, organics, moist.
		3				ALLUVIUM
40		3	12"/24"	36.0		
		4				
45		2	S13	39.0		Loose gray silty fine to coarse SAND, trace organics, moist.
		2				
50		4	23"/24"	40.0		
		4				
55		2	S14	44.0		Loose gray brown fine to medium sandy SILT, little clay, organics, moist.
		4				
60		5	22"/24"	46.0		
		4				
65		2	S15	49.0		Same.
		2				
70		3	22"/24"	51.0		
		3				
75		2	S16	54.0		Same.
		2				
80		3	23"/24"	56.0		
		3				
85		2	S17	59.0		Same.
		2				
90		3	22"/24"	61.0		
		3				
95		4	S18	64.0		Medium dense gray brown fine to medium sandy SILT, little clay, organics, moist.
		4				
100		7	22"/24"	66.0		
		7				
105		7	S19	69.0		Bottom of Exploration at 71.0 ft.
		10				
110		10	17"/24"	71.0		

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks		
		2	S6	29.0		Loose gray fine to medium sandy SILT, organics, wet. ALLUVIUM Bottom of Exploration at 31.0 ft.		
		2	24"/24"	31.0				
		4						
		5						
35								
40								
45								
50								
55								
60								
65								
70								
					FILE NO.	70819-000	BORING NO.	HA-104

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
		1	S7	30.0		Loose gray brown sandy SILT, organics.
		3				
		3				ALLUVIUM
		6		32.0		Bottom of Exploration at 32.0 ft.
35						
40						
45						
50						
55						
60						
65						
70						
					FILE NO.	70819-000
					BORING NO.	HA-105

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	26-May-00
DRILLER	L. TODD	DATE FINISHED	26-May-00

Elevation	266.08	ft	Datum	City	Boring Location	See Boring Location Plan		
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount			Drill Mud
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type	<input type="checkbox"/> Bentonite
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Polymer
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> None
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						ASPHALT
			S1	0.5	0.5	Medium dense black brown fine to coarse SAND, some gravel, dry.
						FILL
			S2	2.0		Medium dense brown fine to coarse SAND, damp.
			S3	4.0		Dense brown black fine to coarse SAND, little silt, brick, damp.
5			S4	6.0		Same, except medium dense.
			S5	8.0		Medium dense brown orange fine to coarse SAND, moist.
			S6	13.0	13.0	Loose brown gray fine sand SILT, trace to little clay, trace organics, moist.
15			S7	18.0		Same.
			S8	23.0	23.0	Very dense gray brown silty SAND, some gravel. Pockets of brown fine to coarse SAND, wet.
			S9	28.0		Same.
25						GLACIAL TILL
30						

Water Level Data				Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O Open End Rod	Overburden (Linear ft) 49.0
26-May		0.5			18	T Thin Wall Tube	Rock Cored (Linear ft) 5.0
						U Undisturbed Sample	Number of Samples 13S
						S Split Spoon Sample	
						G Geoprobe	
						BORING NO.	HA-107

CORE BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISE	DATE STARTED	26-May-00
DRILLER	L. TODD	DATE FINISHED	26-May-00

Elevation		ft	Datum		Boring Location			
Item	Casing	Sampler	Core Barrel	Rig Make & Model				Drill Mud
Type	HAS	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type	<input type="checkbox"/> Bentonite
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Polymer
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> None
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Drilling Rate (min/ft)	Core No. Depth (ft)	Recovery RQD		Weathering	Stratum Change (ft)	Visual Classification and Remarks
			(in)	(%)			
		49.5					0-2 ft. Highly fractured.
	Avg. 4 ft. per minute		1.9/5.0	38			Competent red sandstone with interbedded gray sandstone.
5		54.5					QUEENSTONE FORMATION
10							
15							
20							
25							
30							

Water Level Data						Sample ID		Summary				
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	T	U	S	G	Overburden (Linear ft)	Rock Cored (linear ft) Samples
											BORING NO.	HA-107

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	12-Jun-00
DRILLER	L. TODD	DATE FINISHED	12-Jun-00

Elevation	251.78	ft	Datum	City	Boring Location	See Boring Location Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount	
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head
						Drill Mud
						<input type="checkbox"/> Bentonite
						<input type="checkbox"/> Polymer
						<input checked="" type="checkbox"/> None
						Casing
						<input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0		7	S1	0.0		Medium dense brown black gray silty coarse to fine SAND, some gravel, ash, dry.
		10				FILL
		7	17"/24"	2.0	2.5	Same.
		7	S2	2.0		Medium dense brown silty coarse to fine SAND, dry.
		8				
		11	14"/24"	4.0		Medium dense brown black silty coarse to fine SAND, moist.
		9	S3	4.0		
5		10				ALLUVIUM
		10	17"/24"	6.0		
		2	S4	9.0		Very loose gray brown fine sandy SILT, little clay, organics, moist.
10		1				
		2	15"/24"	11.0		
		1	S5	14.0		Very loose, gray brown silty medium to fine SAND, organics, moist.
15		2				
		2	16"/24"	16.0		
		1	S6	19.0		Same.
20		1				
		2	20"/24"	21.0		
		2	S7	24.0	25.0	Same.
25		1				
		30	16"/24"	26.0		Very dense red silty fine to coarse SAND, dry.
		46				DISINTEGRATED RED SANDSTONE
		100/3	S8	27.5		Same, except some rock fragment.
			4"/4"	27.8		Bottom of Exploration at 27.9 ft.
						Auger Refusal.
30						

Water Level Data						Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	T	U	S
						Open End Rod	Thin Wall Tube	Undisturbed Sample	Split Spoon Sample
						Geoprobe			
								Overburden (Linear ft)	27.8
								Rock Cored (Linear ft)	--
								Number of Samples	8S
								BORING NO.	HA-109

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	23-May-00
DRILLER	L. TODD	DATE FINISHED	23-May-00

Elevation	251.83	ft	Datum	City	Boring Location	See Boring Location Plan			
Item	Casing	Sampler	Core Barrel	Rig Make & Model			CME 55 - Truck Mount		Drill Mud
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type		<input type="checkbox"/> Bentonite
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Polymer	
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> None	
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven	<input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0			No sample		0.5	ASPHALT
					1.0	CRUSHED STONE
	3	S1	1.0			Loose gray brown silty fine to coarse SAND, pocket of black fine to coarse SAND, dry.
	3	5"/12"	2.0			Medium dense gray brown fine to coarse SAND, little silt, wet.
	5	S2	2.0			FILL
	6					
	7					
	6	8"/24"	4.0			Medium dense gray brown green (mottled) silty fine to coarse SAND, some fine gravel, wood, moist. Water in borehole at 3.9 ft.
	4	S3	4.0			
5	19					
	21	12"/24"	6.0			Dense gray brown gravelly SAND, wet. Rock Obstruction in shoe.
	19	S4	6.0			
	27					
	32	16"/24"	8.0			Very dense gray brown fine to coarse SAND, some fine gravel, wet.
	21	S5	8.0			
	24					
	26					
10	25	20"/24"	10.0		10.0	Medium dense gray brown fine to coarse SAND.
	15	S6	10.0			
	14					ALLUVIUM
	19	17"/24"	12.0			
15	4					
	6	S7	15.0			Loose gray brown fine to coarse SAND, some fine to coarse gravel, moist.
	3					
	3	14"/24"	17.0			
20	1					
	1	S8	20.0			Very loose gray brown fine sand SILT, wood, natural laminations in soils, moist.
	2					
	2	18"/24"	22.0			
25	1					
	1	S9	25.0			Very loose gray brown fine sand SILT, little clay, wood.
	1					
	1	24"/24"	27.0			
30						

Water Level Data						Sample ID		Summary					
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	T	U	S	G	Overburden (Linear ft)	Rock Cored (Linear ft)	Number of Samples
23-May	10	0.75			3.9						58.5	5	14S
											BORING NO.	HA-111	

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
		1	S10	30.0		Very loose gray brown fine sand SILT, little clay, root structures, wood, moist.
		2				
		2				
		3	24"/24"	32.0		
						ALLUVIUM
35		2	S11	35.0		Same, except some clay.
		2				
		2				
		2	24"/24"	37.0		
40		2	S12	40.0		Very loose gray-green fine sand SILT, root structures, red fine to coarse sand in shoe, moist.
		2				
		6	24"/24"	42.0	42.0	
						GLACIAL TILL
45		70	S13	45.0		Dense red brown SILT, little clay, gray green fractured sandstone.
		33				
		8			46.0	
		12	16"/24"	47.0		
50		100/2				No Recovery.
55		100/2	S14	55.0	55.0	Very dense red, brown fractured sandstone, red brown silt, wet.
			2"/3"	55.3		
						WEATHERED BEDROCK
						Auger Refusal at 58.5 ft.; began rock coring.
60						
65						
70						
					FILE NO.	70819-000
					BORING NO.	HA-111

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
						Competent red sandstone with interbedded gray sandstone.
						QUEENSTON FORMATION
						Bottom of Exploration at 63.5 ft.
65						Monitoring well installed in adjacent borehole. See Installation Report for LBA-MWI
70						
75						
80						
85						
90						
95						
##						
					FILE NO.	70819-000
					BORING NO.	HA-111

CORE BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISE	DATE STARTED	23-May-00
DRILLER	L. TODD	DATE FINISHED	23-May-00

Elevation		ft		Datum		Boring Location					
Item	Casing	Sampler	Core Barrel	Rig Make & Model						Drill Mud	
Type	HAS	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type		<input type="checkbox"/> Bentonite		
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Polymer			
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> None			
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing		<input type="checkbox"/> Driven <input type="checkbox"/> Spun		

Depth (ft)	Drilling Rate (min/ft)	Core No.	Recovery RQD		Weathering	Stratum Change (ft)	Visual Classification and Remarks
			(in)	(%)			
		58.5					
5							Competent red sandstone with interbedded gray sandstone.
5							QUEENSTONE FORMATION
3							
4							
5		63.5					
10							
15							
20							
25							
30							

Water Level Data						Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	T	Overburden (Linear ft)	
								Rock Cored (linear ft)	
								Samples	
								BORING NO. HA-111	

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	9-Jun-00
DRILLER	L. TODD	DATE FINISHED	9-Jun-00

Elevation	260.89	ft	Datum	City	Boring Location	See Boring Location Plan			
Item	Casing	Sampler	Core Barrel	Rig Make & Model			CME 55 - Truck Mount		Drill Mud
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type		<input type="checkbox"/> Bentonite
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Polymer	<input type="checkbox"/> None
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> None	<input type="checkbox"/> Spun
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven	<input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						CRUSHED STONE FILL
	3		S1	0.0		
	4				1.0	
	4					Loose brown silty fine to coarse SAND, dry.
	3	3	14"/24"	2.0		
	3		S2	2.0		Loose brown silty fine to coarse SAND, trace clay, trace organics, moist.
	3					ALLUVIUM
	3	3	18"/24"	4.0		
	3		S3	4.0		Same, except very loose.
5	1					
	2					
	3		18"/24"	6.0		
	5		S4	9.0		Medium dense brown fine sandy SILT, some clay pockets, moist.
10	5					
	6		17"/24"	11.0		
	2		S5	14.0		Loose gray brown fine sandy SILT, some clay pockets, moist.
15	2					
	3					
	5		20"/24"	16.0		
	2		S6	19.0		Loose gray brown silty coarse to fine SAND, some gravel, moist.
20	4					GLACIAL TILL
	5					
	6		12"/24"	21.0		
	3		S7	24.0		Loose gray brown silty fine to coarse SAND, some gravel, little clay, wet.
25	5					
	4					
	5		24"/24"	26.0		
30						

Water Level Data						Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	Open End Rod	Overburden (Linear ft)	41
						T <th>Thin Wall Tube</th> <th>Rock Cored (Linear ft)</th> <td>--</td>	Thin Wall Tube	Rock Cored (Linear ft)	--
						U <th>Undisturbed Sample</th> <th>Number of Samples</th> <td>10S</td>	Undisturbed Sample	Number of Samples	10S
						S <th>Split Spoon Sample</th> <td colspan="2">BORING NO. HA-112</td>	Split Spoon Sample	BORING NO. HA-112	
						G <th>Geoprobe</th> <td colspan="2"></td>	Geoprobe		

TEST BORING REPORT

BORING NO.

HA-113a

Page 2 of 2

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	7-Jun-00
DRILLER	L. TODD	DATE FINISHED	8-Jun-00

Elevation	270.8	ft	Datum	City	Boring Location	See Boring Location Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount	
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head
						Hammer Type
						<input checked="" type="checkbox"/> Safety
						<input type="checkbox"/> Doughnut
						Casing
						<input type="checkbox"/> Driven
						<input type="checkbox"/> Spun
						Drill Mud
						<input type="checkbox"/> Bentonite
						<input type="checkbox"/> Polymer
						<input checked="" type="checkbox"/> None

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						(Blind augered to 10.0 ft.) See Boring HA-113
5						
10		1	S5	10.0		Loose brown red silty fine to coarse SAND, little rock fragments, slag, moist.
		2				
		3				
		3	3"/24"	12.0		FILL
15		7	S6	15.0		(Slag obstruction in spoon)
		11				
		11				
		14	1"/24"	17.0		
20		3	S7	20.0	20.0	Very dense gray brown silty fine to coarse SAND, some gravel, pockets of clayey silt, moist.
		14				
		36				
		50	22"/24"	22.0		GLACIAL TILL
25		30	S8	25.0		Same as above.
		76				
		98				
		100/3	22"/24"	27.0		Bottom of Exploration at 27.0 ft.
30						

Water Level Data						Sample ID	Summary
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)		
						O Open End Rod	Overburden (Linear ft) 27.0 ft.
						T Thin Wall Tube	Rock Cored (Linear ft) --
						U Undisturbed Sample	Number of Samples 8S
						S Split Spoon Sample	
						G Geoprobe	
						BORING NO.	HA-113a

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	2-Jun-00
DRILLER	L. TODD	DATE FINISHED	2-Jun-00

Elevation	252.4	ft	Datum	City	Boring Location	See Boring Location Plan		
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount			Drill Mud
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type	<input type="checkbox"/> Bentonite
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Polymer
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> None
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0		4	S1	0.0		0.4 ft. TOPSOIL
		16				Medium dense black blue silty fine to coarse SAND, slag, dry.
		19				FILL
		24	11"/24"	2.0		
		22	S2	2.0		Same, except wet.
		12				
		21	12"/24"	4.0		
		42	S3	4.0		Same.
		25				
5		10				
		20	8"/24"	6.0		Medium dense brown fine to coarse SAND, slag.
		14	S4	6.0		
		12				
		4				
		2	10"/24"	8.0		
		3	S5	8.0		Medium dense gray brown fine to coarse SAND, some gravel, wet.
		5				
		8				
10		10	8"/24"	10.0		
		18	S6	10.0		Same.
		7				
		4				
		6	8"/24"	12.0		ALLUVIUM
15		1	S7	15.0		Loose gray brown fine sand SILT, organics, moist.
		2				
		3				
		3	3"/24"	17.0		
20		2	S8	20.0		Very loose gray brown fine sand SILT, little clay, organics, moist.
		1				
		3				
		3	16"/24"	22.0		
25		1	S9	25.0		Same.
		1				
		2				
		3	18"/24"	27.0		Bottom of Exploration at 27.0 ft.
30						

Water Level Data					Sample ID	Summary
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	
						O Open End Rod
						T Thin Wall Tube
						U Undisturbed Sample
						S Split Spoon Sample
						G Geoprobe
						Overburden (Linear ft) 27
						Rock Cored (Linear ft) --
						Number of Samples 9S
						BORING NO. HA-116

TEST BORING REPORT

BORING NO.

HA-118

Page 1 of 2

PROJECT PORT OF ROCHESTER
 LOCATION ROCHESTER, NEW YORK
 CLIENT LABELLA ASSOCIATES
 CONTRACTOR GEOLOGIC ENTERPRISES
 DRILLER L. TODD

H&A FILE NO. 70819-000
 PROJECT MGR. M. VALENTINE
 FIELD REP. R. DEDRICK
 DATE STARTED 8-Jun-00
 DATE FINISHED 8-Jun-00

Elevation	242.78	ft	Datum	City	Boring Location	See Boring Location Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount	Drill Mud
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck <input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	<input type="checkbox"/> Bentonite
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV <input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input type="checkbox"/> Polymer
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track <input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input checked="" type="checkbox"/> Safety
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid <input type="checkbox"/>	<input type="checkbox"/> Cutting Head	<input type="checkbox"/> Doughnut
						<input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						ASPHALT
	9		S1	0.5		Medium dense black brown red silty fine to coarse SAND, brick, some rock fragments, dry.
	18				2.0	FILL
	7	9	13"/18"	2.0	2.0	Medium dense brown silty fine to coarse SAND, moist.
	12		S2			ALLOVIUM
	9					
	8		12"/24"	4.0		
	4		S3	4.0		Loose gray brown silty fine to medium SAND, organics, moist.
	4					
5		3				
		3	12"/24"	6.0		
10		6		10.0		Medium dense gray fine to coarse SAND, little silt, little gravel, wet.
	14		S4			
	12					
		4	16"/24"	12.0		
15		3		15.0		Very loose brown organic SILT, moist.
	1		S5			
	1					
		4	16"/24"	17.0		
					20.0	
20		4		20.0		Medium dense gray brown silty fine to coarse SAND, some gravel, moist.
	16		S6			
	19					GLACIAL TILL
		22	20"/24"	22.0		
25		28		25.0		Very dense brown silty fine to coarse SAND, some gravel, moist.
	100/4		S7			
			10"/10"	25.9		
30						

Water Level Data						Sample ID	Summary
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)		
5/8/2000		0.5			9.1	O Open End Rod T Thin Wall Tube U Undisturbed Sample S Split Spoon Sample G Geoprobe	Overburden (Linear ft) 51 Rock Cored (Linear ft) -- Number of Samples 12S
						BORING NO.	HA-118

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
		100/4	S8 3"/5"	30.0 30.4		Very dense brown silty fine to coarse SAND, some gravel, moist.
35		18 77 100/5	S9 12"/18"	35.0 36.5		Same, except gray brown.
40		36 66 98 87	S10 12"/24"	40.0 42.0		Same.
45		100/5	S11 3"/6"	45.0 45.5		Same, except trace rock fragments.
50		100/4	S12 4"/5"	50.0 50.5		Very dense red silty sandstone rock fragments.
						Bottom of Exploration at 51.0 ft.
55						
60						
65						
70						

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	2-Jun-00
DRILLER	L. TODD	DATE FINISHED	2-Jun-00

Elevation	250.52	ft	Datum	City	Boring Location	See Boring Location Plan		
Item	Casing	Sampler	Core Barrel	Rig Make & Model			Drill Mud	
Type	HSA	SS	NX	CME 55 - Truck Mount				
Inside Diameter (in)	3-1/4	1-3/8	2	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type	
Hammer Weight (lb)	--	140		<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	
Hammer Fall (in)	--	30		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	
				<input type="checkbox"/> Skid		<input type="checkbox"/> Cutting Head	Casing <input type="checkbox"/> Driven <input type="checkbox"/> Spun	

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						ASPHALT
	44		S1	1.0		Very dense brown gravelly fine to coarse SAND, dry.
	66		3 3/12"	2.0		
	10		S2	2.0		FILL Medium dense brown silty fine to medium SAND, little silt, wet.
	10					ALLOVIUM
	6		14"/24"	4.0		Medium dense brown gray, fine to coarse SAND, little silt, wet.
	7		S3	4.0		
5	7					
	7		16"/24"	6.0		Medium dense gray brown fine to coarse SAND, some silt, little rock fragments, wet.
	8		S4	6.0		
	12					
	20		10"/24"	8.0		Medium dense gray brown gravelly fine to coarse SAND, trace silt, wet.
	5		S5	8.0		
	14					
	25		20"/24"	10.0		
10						
	14		S6	14.0		Loose gray brown sandy SILT, wet.
15	3					
	3		18"/24"	16.0		
	3		S7	19.0		Loose gray brown silty fine to coarse SAND, trace gravel, wet.
20	3					
	3		20"/24"	21.0		
	6		S8	24.0		Loose gray brown fine to medium sandy SILT, trace clay, organics, moist.
25	2					
	3		15"/24"	26.0		
	1		S9	29.0		Same.
30	1					
	3		14"/24"	31.0		

Water Level Data						Sample ID	Summary		
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)		Overburden (Linear ft)	51	
						O	Open End Rod	Rock Cored (Linear ft)	--
						T	Thin Wall Tube	Number of Samples	13S
						U	Undisturbed Sample		
						S	Split Spoon Sample		
						G	Geoprobe		
							BORING NO.	HA-119	

TEST BORING REPORT

BORING NO.
HA-119

Page 2 of 2

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
				31.0		
35		1 2 3	S10 17"/24"	34.0 36.0		Loose gray brown fine sand SILT, trace clay, organics, moist.
40		1 2 3	S11 22"/24"	39.0 41.0		Same.
45		1 2 3	S12 24"/24"	44.0 46.0		Loose, gray fine sand SILT, trace clay, organics.
50		1 2 4	S13 18"/24"	49.0 51.0		Same.
						Bottom of Exploration at 51.0 ft.
55						
60						
65						
70						

FILE NO. 70819-000 BORING NO. HA-119

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
		1	S11	30.0		Very loose gray brown fine to medium sandy SILT, trace clay, organics, moist.
		2				
		2	18"/24"	32.0		
						ALLUVIUM
35		1	S12	35.0		Same.
		2				
		2	24"/24"	37.0		
40		1	S13	40.0		Same.
		3				
		3	24"/24"	42.0		
45		1	S14	45.0		Same.
		2				
		4				
		3	22"/24"	47.0		
50		H	S15	50.0		Same.
		2				
		2	24"/24"	52.0		Bottom of Exploration at 52.0 ft.
55						
60						
65						
70						

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	30-May-00
DRILLER	L. TODD	DATE FINISHED	30-May-00

Elevation	276 +/-	ft	Datum	City	Boring Location	See Boring Location Plan		
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount			Drill Mud
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type	
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Bentonite
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> Polymer
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0		5	S1	0.0		Medium dense gray brown fine sand SILT, little clay, dry.
		8				FILL
		14	16"/24"	2.0		Dense gray red fine sand SILT, little clay, dry.
		20	S2	2.0		
		22				
		24	16"/24"	4.0	4.0	Dense brown silty fine to medium SAND, little clay, moist.
		14	S3	4.0		
5		20				ALLUVIUM
		24				
		20	18"/24"	6.0	6.0	Dense brown silty fine SAND, some clay, moist.
		14	S4	6.0		
		21				
		24				
		20	23"/24"	8.0		Dense brown silty fine to medium SAND, little clay, moist.
		19	S5	8.0		
		24				
		26				
10		14	20"/24"	10.0	10.0	Medium dense brown silty fine to coarse SAND, pockets of clay, moist.
		8	S6	10.0		
		12				
		17	19"/24"	12.0		
15		7	S7	15.0		Medium dense brown silty fine to coarse SAND, little clay, moist.
		9				
		9				
		10	16"/24"	17.0		
20		4	S8	20.0		Same.
		5				
		5				
		6	18"/24"	22.0		
25		3	S9	25.0		Loose gray brown sandy SILT, some clay, moist.
		3				
		4				
		4	20"/24"	27.0		
30						

Water Level Data						Sample ID		Summary					
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	T	U	S	G	Overburden (Linear ft)	Rock Cored (Linear ft)	Number of Samples
											61	10	16S
										BORING NO.	HA-121		

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
		2	S10	30.0		
		6				Medium dense brown gray silty fine to coarse SAND, little gravel, wet.
		8				
		10	20"/24"	32.0		GLACIAL TILL
35		2	S11	35.0		Same.
		16				
		18				
		20	14"/24"	37.0		
40		5	S12	40.0		Very dense brown gray silty fine to coarse SAND, some gravel, wet.
		64				
		37				
		67	17"/24"	42.0		
45		14	S13	45.0		Same, except little gravel.
		44				
		1007.4	17"/18"	46.5		
50		6	S14	50.0		Same.
		1007.2	6"/8"	50.7		
55		30	S15	55.0		Same.
		1007.3	3"/9"	55.8		
60		1007.4	S16	60.0	60.4	Same.
			2"/5"	60.4		Began Rock Coring at 61.0 ft.
						Moderately fractured red SANDSTONE with interbedded gray sandstone, clay pockets.
65						BEDROCK
70						Bottom of Exploration at 71.0 ft.
						FILE NO. 70819-000 BORING NO. HA-121

CORE BORING REPORT

BORING NO.

HA-121

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PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISE	DATE STARTED	31-May-00
DRILLER	L. TODD	DATE FINISHED	31-May-00

Elevation	ft	Datum	Boring Location			
Item	Casing	Sampler	Core Barrel	Rig Make & Model		Drill Mud
Type	HAS	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head
						Hammer Type <input checked="" type="checkbox"/> Safety <input type="checkbox"/> Doughnut Casing <input type="checkbox"/> Driven <input type="checkbox"/> Spun
						<input type="checkbox"/> Bentonite <input type="checkbox"/> Polymer <input checked="" type="checkbox"/> None

Depth (ft)	Drilling Rate (min/ft)	Core No. Depth (ft)	Recovery RQD		Weathering	Stratum Change (ft)	Visual Classification and Remarks
			(in)	(%)			
		61.0					
	6 to 7 minutes per foot		4.8	2.1			Moderately fracture red SANDSTONE with interbedded gray sandstone, clay pockets.
							QUEENSTONE FORMATION
5		66.0					
	5 to 5 minutes per foot		4.5	3.9			
10		71.0					Bottom of Exploration at 71.0 ft.
15							
20							
25							
30							

Water Level Data						Sample ID	Summary
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O Open End Rod T Thin Wall Tube U Undisturbed Sample S Split Spoon Sample G Geoprobe	Overburden (Linear ft) 61 Rock Cored (linear ft) 10 Samples 16S
							BORING NO. HA-121

TEST BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	D. NOSTRANT
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	31-May-00
DRILLER	L. TODD	DATE FINISHED	31-May-00

Elevation	252.8	ft	Datum	City	Boring Location	See Boring Location Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME-55 Truck Mount	
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Track <input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type
Inside Diameter (in)	3-1/4	1-3/8	1-7/8	<input type="checkbox"/> ATV <input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track <input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid <input type="checkbox"/>	<input checked="" type="checkbox"/> Cutting Head	Casing <input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0					0.3	TOPSOIL Medium dense dark brown, coarse to fine sand, little cinders, little gravel.
			S1	0.0		
						FILL
			S2	2.0		Same.
			S3	4.0		No Recovery.
5			S4	6.0		Loose dark brown coarse to fine sand, some gravel, trace silt, wet.
			S5	8.0		Same.
10			S6	10.0		Same.
			S7	12.0		
			S8	14.0		Dense blue-gray gravel, little coarse to fine sand, wet.
15				14.3		Very loose brown ORGANICS, trace sand, trace silt, wet.
				15.5		Very loose gray-brown fine clayey SILT, some sand, little organics, moist.
						ALLUVIUM
20			S9	20.0		Same, except little fine sand.
				22.0		
25			S10	25.0		Same.
				27.0		
30						

Water Level Data						Sample ID	Summary
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)		
						O Open End Rod	Overburden (Linear ft) 37
						T Thin Wall Tube	Rock Cored (Linear ft) 5
						U Undisturbed Sample	Number of Samples 12S
						S Split Spoon Sample	
						G Geoprobe	
						BORING NO.	HA-122

TEST BORING REPORT

BORING NO.

HA-123

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PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	5-Jun-00
DRILLER	L. TODD	DATE FINISHED	6-Jun-00

Elevation	253.64	ft	Datum	City	Boring Location	See Boring Location Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount	
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck <input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV <input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track <input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid <input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing <input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						(0.3 ft. TOPSOIL)
	4		S1	0.0		Medium dense brown gray sandy SILT, little coarse gravel, dry.
	8					FILL
	8		8"/24"	2.0		
	8		S2	2.0		Medium dense brown red silty fine to coarse SAND, trace fine gravel, dry.
	7					
	8		13"/24"	4.0		
	5		S3	4.0		Same, except moist.
5	4					
	3		16"/24"	6.0		
	2		S4	6.0		Loose brown red silty fine to coarse SAND, trace fine gravel, wet.
	2					
	2		20"/24"	8.0		
	1		S5	8.0		Medium dense black brown silty fine to coarse SAND, wood, wet.
	4					
	8		16"/24"	10.0		
10	5		S6	10.0		No Recovery.
	5					
	2		0"/24"	12.0		No Recovery.
	5		S7	12.0		No Recovery.
	5					
	5		0"/24"	14.0		
	5		S8	14.0		Loose gray brown silty fine to coarse SAND, some organics, moist.
15	4					
	1		19"/24"	16.0		ALLUVIUM
	3					
	2		S9	19.0		Loose gray brown clayey SILT, little sand, moist.
20	2					
	2		10"/24"	21.0		
	2					
	1		S10	24.0		Same, except little clay.
25	2					
	2		14"/24"	26.0		
	2					
	2		S11	29.0		Same.
30	2					
	4		15"/24"	31.0		

Water Level Data						Sample ID	Summary
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O Open End Rod	Overburden (Linear ft) 114
						T Thin Wall Tube	Rock Cored (Linear ft) 2
						U Undisturbed Sample	Number of Samples 24S
						S Split Spoon Sample	
						G Geoprobe	
							BORING NO. HA-123

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
				31.0		
35		2	S11	34.0		Very loose gray brown fine to medium sand SILT, trace clay, organics, moist.
		2				
		2				
		4	20"/24"	36.0		
						ALLUVIUM
40		1	S12	39.0		Same.
		2				
		2				
		3	14"/24"	41.0		
45		1	S13	44.0		Very loose gray silty fine to medium SAND, moist.
		2				
		2				
		3	19"/24"	46.0		
50		1	S14	49.0		Loose gray fine sand SILT, trace clay, organics, moist.
		2				
		3				
		4	20"/24"	51.0		
55		1	S15	54.0		Same.
		1				
		3				
		3	20"/24"	56.0		
60		1	S16	59.0		Same.
		2				
		5				
		4	20"/24"	61.0		
65		3	S17	64.0		Loose gray fine sand SILT, trace clay organics, moist.
		1				
		4				
		5	24"/24"	66.0		
70		WOH	S18	69.0		Same, except medium dense.
		5				
		7				
		8	22"/24"	71.0		

TEST BORING REPORT

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
75		4	S19	74.0		Medium dense gray fine sandy SILT, trace clay, organics moist.
		5				ALLUVIUM
80		8	22"/24"	76.0		
		9				
85		2	S20	79.0		Same.
		5				
90		7	23"/24"	81.0		
		9				
85		5	S21	84.0		Same.
		5				
90		8	20"/24"	86.0		
		9				
95		5	S22	89.0		Medium dense gray brown silty medium to fine SAND, trace clay, moist.
		5				
100		8	21"/24"	91.0		
		9				
105		WOR	S23	94.0		Very loose gray brown silty medium to fine SAND, trace clay, moist.
		WOR				
110		WOR	22"/24"	96.0		
		5				
105		7	S24	99.0		Same, except medium dense.
		8				
110		9	22"/24"	101.0		
		12				
105		WOR	S25	104.0		Same, except very loose.
		WOR				
110		WOR	24"/24"	106.0		
		WOR				
110		3	S26	109.0		Medium dense gray brown silty fine to medium SAND, trace clay, pockets of rock fragments, moist.
		5				
110		5				
		12	23"/24"	111.0		
			FILE NO.	70819-000	BORING NO. HA-123	

TEST BORING REPORT

BORING NO.
HA-123

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Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks		
						ALLOVIUM		
115		1007.2	S27 2 2/3"	114.0 114.2	114.0	Very dense sandy ROCK FRAGMENTS. WEATHERED BEDROCK Began rock coring 114.0 ft. Bottom of Exploration at 116.0 ft.		
120								
125								
130								
135								
140								
145								
150								
					FILE NO.	70819-000	BORING NO.	HA-123

CORE BORING REPORT

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISE	DATE STARTED	5-Jun-00
DRILLER	L. TODD	DATE FINISHED	6-Jun-00

Elevation	253.6	ft	Datum	City	Boring Location	See Boring Location Plan			
Item	Casing	Sampler	Core Barrel	Rig Make & Model			CNE 55 - Truck Mount		
Type	HAS	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head	Hammer Type		
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch	<input checked="" type="checkbox"/> Safety	<input type="checkbox"/> Polymer	
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit	<input type="checkbox"/> Doughnut	<input checked="" type="checkbox"/> None	
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head	Casing	<input type="checkbox"/> Driven <input type="checkbox"/> Spun	

Depth (ft)	Drilling Rate (min/ft)	Core No. Depth (ft)	Recovery RQD		Weathering	Stratum Change (ft)	Visual Classification and Remarks
			(in)	(%)			
		114.0				114.0	Begin coring at 114.0 ft.
115		R1	21 15	88 63	MOD		Moderately soft, moderately weathered red-brown-gray mottled fine-grained SANDSTONE.
		116.0				116.0	QUEENSTON FORMATION
							Bottom of Boring at 116.0 ft.
120							
125							
130							
135							
140							

Water Level Data						Sample ID		Summary	
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)	O	Open End Rod	Overburden (Linear ft)	114
						T	Thin Wall Tube	Rock Cored (linear ft)	2
						U	Undisturbed Sample	Samples	27S
						S	Split Spoon Sample		
						G	Geoprobe		
								BORING NO.	HA-123

TEST BORING REPORT

BORING NO.
HA-125

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PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISES	DATE STARTED	12-Jun-00
DRILLER	L. TODD	DATE FINISHED	12-Jun-00

Elevation	255.29	ft	Datum	City	Boring Location	See Boring Location Plan
Item	Casing	Sampler	Core Barrel	Rig Make & Model	CME 55 - Truck Mount	
Type	HSA	SS	NX	<input checked="" type="checkbox"/> Truck	<input type="checkbox"/> Tripod	<input checked="" type="checkbox"/> Cat-Head
Inside Diameter (in)	3-1/4	1-3/8	2	<input type="checkbox"/> ATV	<input type="checkbox"/> Geoprobe	<input type="checkbox"/> Winch
Hammer Weight (lb)	--	140		<input type="checkbox"/> Track	<input type="checkbox"/> Air Track	<input type="checkbox"/> Roller Bit
Hammer Fall (in)	--	30		<input type="checkbox"/> Skid	<input type="checkbox"/>	<input type="checkbox"/> Cutting Head
						Drill Mud
						<input type="checkbox"/> Bentonite
						<input type="checkbox"/> Polymer
						<input checked="" type="checkbox"/> None
						Casing <input type="checkbox"/> Driven <input type="checkbox"/> Spun

Depth (ft)	Casing Blows per ft	Sampler Blows per 6 in	Sample Number & Recovery	Sample Depth (ft)	Stratum Change (ft)	Visual Classification and Remarks
0						ASPHALT CRUSHED STONE
	12	7	S1 4"/12"	1.0	2.0	Medium dense brown black gravelly SAND, slag, dry
	3	4	S2	2.0		Loose brown gray silty fine to medium SAND, damp FILL
	2	7	15"/24"	4.0		Same.
	3	2	S3	4.0		
5		4				
	2	2	14"/24"	6.0	6.0	Loose gray silty fine SAND, some clay, little gravel, damp
	3	3	S4	6.0		
	3	7	18"/24"	8.0	8	Very dense brown gray silty fine SAND, some gravel, damp
	8	28	S5	8.0		
	28	29				GLACIAL TILL
10		28	15"/24"	10.0		Bottom of Exploration at 10.0 ft.
15						
20						
25						
30						

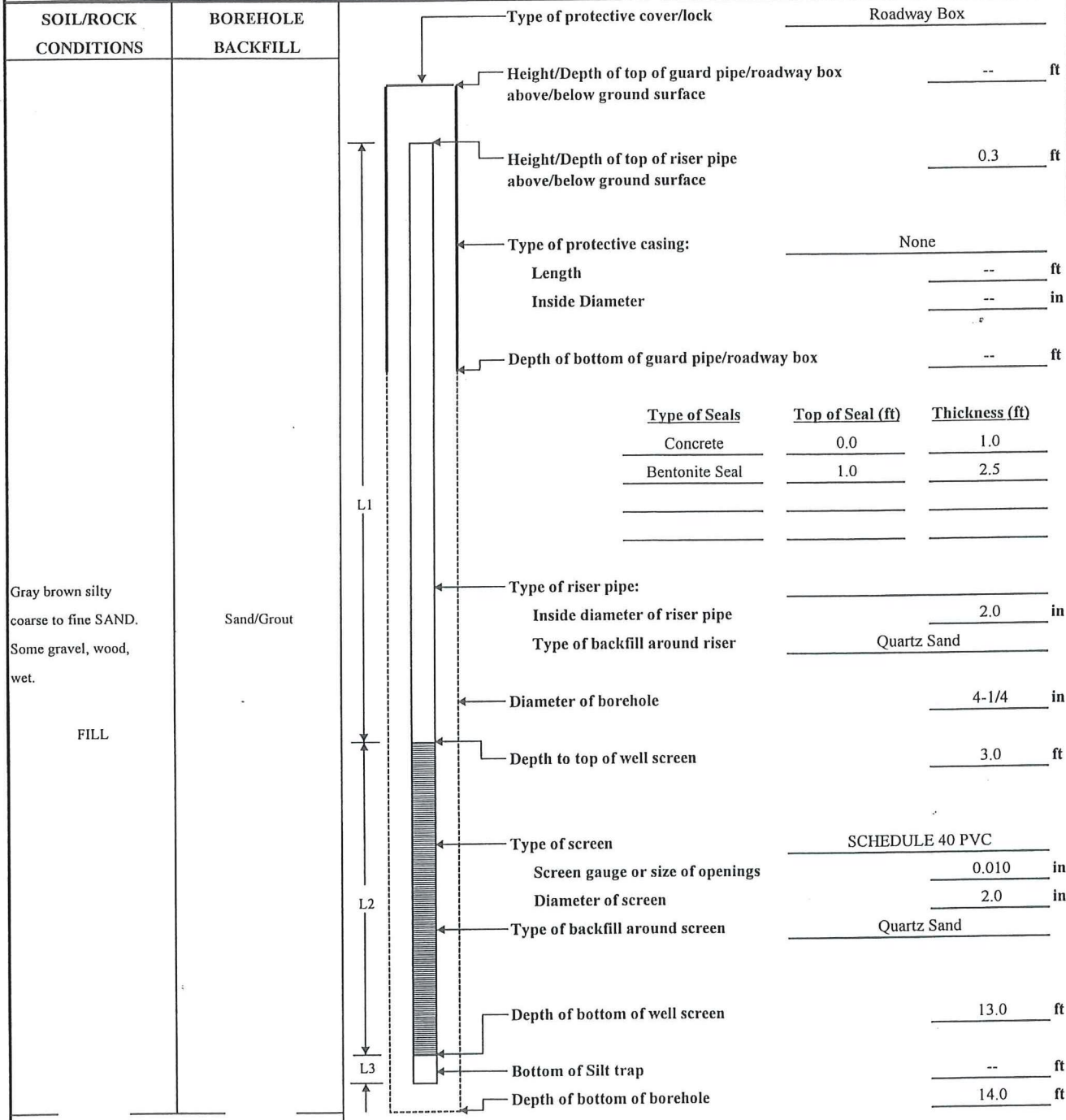
Water Level Data					Sample ID		Summary		
Date	Time	Elapsed Time (hrs)	Bottom of Casing (ft)	Bottom of Boring (ft)	Water (ft)				
						O	Open End Rod	Overburden (Linear ft)	10
						T	Thin Wall Tube	Rock Cored (Linear ft)	--
						U	Undisturbed Sample	Number of Samples	5S
						S	Split Spoon Sample	BORING NO. HA-125	
						G	Geoprobe		

OBSERVATION WELL INSTALLATION REPORT

Well No.
LBA-MW1
Boring No.
HA-111*

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISE	DATE INSTALLED	5/24/2000
DRILLER	L. TODD	WATER LEVEL	

Ground El. <u>251.83</u> ft	Location <u>N: 1188376.32</u>	<input type="checkbox"/> Guard Pipe
El. Datum <u>City</u>	<u>E: 1408 396.58</u>	<input checked="" type="checkbox"/> Roadway Box



SOIL/ROCK CONDITIONS: Gray brown silty coarse to fine SAND. Some gravel, wood, wet.

BOREHOLE BACKFILL: Sand/Grout

FILL: (Bottom of Exploration) (Numbers refer to depth from ground surface in feet)

(Not to Scale)

$$\frac{3}{\text{Riser Pay Length (L1)}} \text{ ft} + \frac{10}{\text{Length of screen (L2)}} \text{ ft} + \frac{0}{\text{Length of silt trap (L3)}} \text{ ft} = \frac{13}{\text{Pay length}} \text{ ft}$$

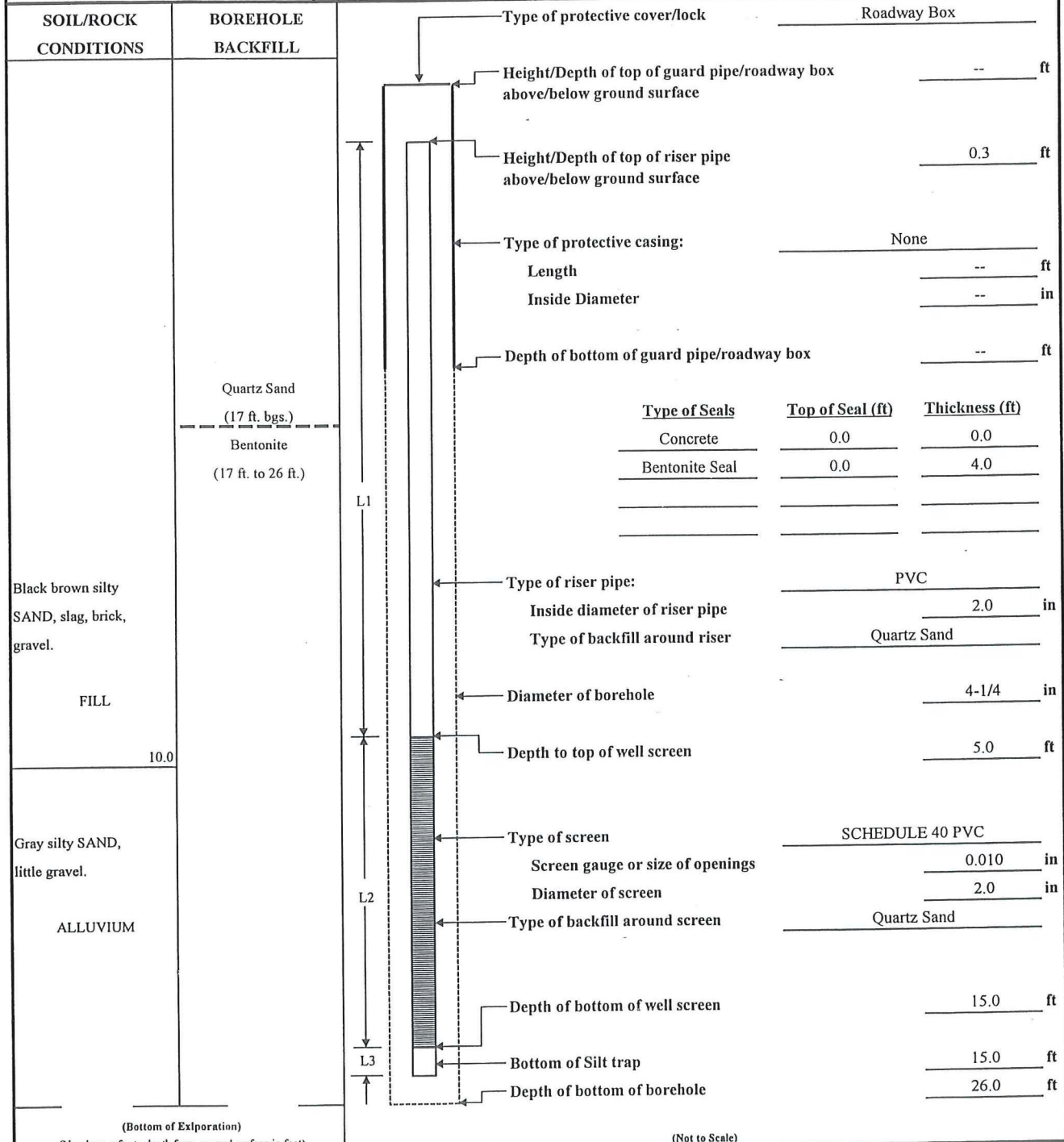
COMMENTS: Well installed 4 ft. west of Boring HA-111. Hole was blind augered to 14.0 ft. per Greg Senegal of Labella Associates

OBSERVATION WELL INSTALLATION REPORT

Well No.
LBA-MW2
Boring No.
HA-117

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISE	DATE INSTALLED	5/24/2000
DRILLER	L. TODD	WATER LEVEL	

Ground El.	253.7	ft	Location	N: 1188222.94	<input type="checkbox"/> Guard Pipe
El. Datum	City		E: 1408074.34		<input checked="" type="checkbox"/> Roadway Box



(Bottom of Exploration) (Numbers refer to depth from ground surface in feet) (Not to Scale)

$$\begin{array}{r}
 \underline{5} \text{ ft} + \underline{10} \text{ ft} + \underline{0} \text{ ft} = \underline{15} \text{ ft} \\
 \text{Riser Pay Length (L1)} \quad \text{Length of screen (L2)} \quad \text{Length of silt trap (L3)} \quad \text{Pay length}
 \end{array}$$

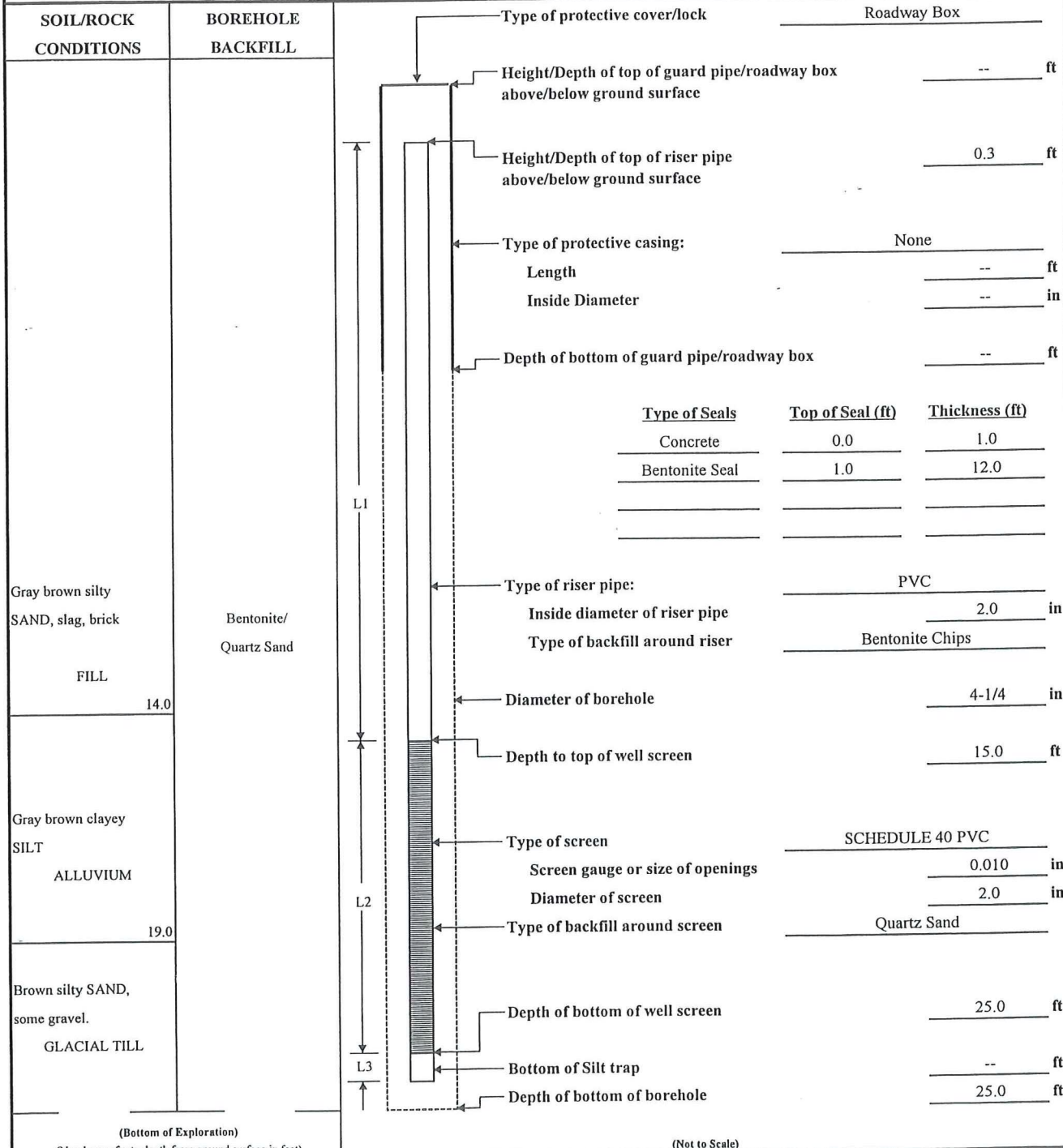
COMMENTS: Bottom of borehole seal from 26.0 ft. to 17.0 ft. b.g.s. using Bentonite Chips.

OBSERVATION WELL INSTALLATION REPORT

Well No.
LBA-MW3
Boring No.
HA-114a

PROJECT	PORT OF ROCHESTER	H&A FILE NO.	70819-000
LOCATION	ROCHESTER, NEW YORK	PROJECT MGR.	M. VALENTINE
CLIENT	LABELLA ASSOCIATES	FIELD REP.	R. DEDRICK
CONTRACTOR	GEOLOGIC ENTERPRISE	DATE INSTALLED	5/25/2000
DRILLER	L. TODD	WATER LEVEL	

Ground El.	261.92	ft	Location	N: 1187851.82	<input type="checkbox"/> Guard Pipe
El. Datum	City		E: 1407798.33		<input checked="" type="checkbox"/> Roadway Box



14.7	ft	+	10	ft	+	0	ft	=	24.7	ft
Riser Pay Length (L1)			Length of screen (L2)			Length of silt trap (L3)			Pay length	

COMMENTS: _____

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APPENDIX B

Results of Laboratory Soil Testing

Haley & Aldrich of New York

Project: Port of Rochester
 Project #: 70819-000
 Client: Labella Associates, P.C.
 Subject: Data Table of Recent Explorations

TABLE B-1 - TEST DATA - RECENT SUBSURFACE INVESTIGATIONS

Date: 23-Jun-00
 Created By: MSV
 Checked By: SEW

PROJECT IDENTIFICATION	EXPLORATION IDENTIFICATION	SURFACE ELEVATION (FT)	LOCATION		DEPTH (FT)	NATURAL WATER CONTENT (%)	LABORATORY TESTS			TOTAL UNIT WEIGHT (PCF)	SHEAR STRENGTH (TSF)			OTHERS
			NORTHING (FT)	EASTING (FT)			ATTERBERG LIMITS (%)	TV	SV Remolded		PP/2			
	HA-101	251.8			10 to 12									4% < #200 98% < #200
	HA-101				60 to 62									
	HA-122	252.8			25 to 27	87.3	105	74	31					80% < #200
	HA-122				30 to 32									
	HA-123	253.6			19 to 21	43.1	45	38	7					86% < #200
	HA-123				24 to 26									

Haley & Aldrich of New York

Project: Port of Rochester
 Project #: 70819-000
 Client: LaBella Associates, P.C.
 Subject: Data Table of Existing Explorations

TABLE B-2 - TEST DATA - EARLIER SUBSURFACE INVESTIGATIONS

Date: 31-Jan-00
 Created By: BEBa
 Checked By: SEW

PROJECT IDENTIFICATION	EXPLORATION IDENTIFICATION	SURFACE ELEVATION (FT)	LOCATION		DEPTH (FT)	NATURAL WATER CONTENT (%)	LABORATORY TESTS			TOTAL UNIT WEIGHT (PCF)	INDEX STRENGTH (TSF)			OTHERS	
			NORTHING (FT)	EASTING (FT)			WL	WP	IP		TV	SV R	PP/2		
Genesee River Crossing H&A #70037 December-89	B-1	222.7	1186728	759540	4.5	39.7				105.3	0.17	0.1	0.13		
						4.7	37.3						0.03		
						5	21.1					0.08	0.14	0.06	
						5.2	39						0.04		LOI = 2.4%
						5.7	34.1	30	24.7	5.3		0.13	0.26	0.28	
						6	41.6					0.17	0.3	0.53	
						6.2	38.6					0.18	0.22	0.16	GRAIN SIZE G = 2.66
						11.1	35.7				103.7	0.15	0.16	0.13	
							39.3						0.03		LOI = 3.0 % GRAIN SIZE
							38.3	34.8	24.2	10.7					
							37.4								
							11.3	39.6			105.2				G = 2.67
							10	43.9				0.2	0.29	0.38	
							10.2	39.9				0.2	0.06	0.17	G = 2.67
							10.5	50.2				0.2	0.2	0.17	
					10.7	40				0.17	0.19	0.28			
					10.9	32.1				0.11	0.24	0.26			
					11.2	49.5				0.11	0.04		LOI = 4.3%		
					11.5	43.5				0.18	0.21	0.18			
						34.8	45.3	26.9	18.4		0.06				
						35.2				0.12	0.22	0.28			
						34.5				0.12	0.05				
						29.8				0.08	0.34	0.18	GRAIN SIZE		
	B-4	239.2	1186695	759892	11.7	34.5				0.08	0.34	0.18			

Haley & Aldrich of New York

Project: Port of Rochester
 Project #: 70819-000
 Client: Labella Associates, P.C.
 Subject: Data Table of Existing Explorations

TABLE B-2 - TEST DATA - EARLIER SUBSURFACE INVESTIGATIONS

Date: 31-Jan-00
 Created By: BEBA
 Checked By: SEW

PROJECT IDENTIFICATION	EXPLORATION IDENTIFICATION	SURFACE ELEVATION (FT)	LOCATION		DEPTH (FT)	NATURAL WATER CONTENT (%)	LABORATORY TESTS			OTHERS			
			NORTHING (FT)	EASTING (FT)			ATTENBERG LIMITS (%)				TOTAL UNIT WEIGHT (PCF)	IDEX STRENGTH (TSP)	
							WL	WP	IP	TV	SV R	PP/2	
Stutson Street Bridge NYSDOT December-97	DA-B-101	275.00	1186450	756550.001									
	DN-B-102	273.13	1186450	756550.001									
	DN-B-103	272.44	1186450	756550.001									
	DN-B-3	216.77	1185260.09	758819.15									
	DN-B-4	216.22	1185182.4	758779.05									
	DN-B-5	225.60	1185115.4	758737.87									
	DN-B-51	219.10	1185214.8	758998.7									
	DN-B-52	220.91	1185179.6	758984.2									
	DN-B-53	221.30	1185141.6	758969.1									
	DN-B-54	222.88	1185069.9	758947.22									
	DN-B-55	226.88	1185019.8	758933.4									
	DN-B-9	251.67	1184912.1	759236.98									
	FH-B-1	289.79	1185295.5	758569.1									
	FH-B-10	256.98	1185181.6	759587.83									
	FH-B-11	256.89	1184939.6	759545.62									
	FH-B-12	254.69	1185137.1	759762.03									
	FH-B-13	253.18	1185162.5	759952.78									
	FH-B-14	251.97	1185098.5	760109.9									
	FH-B-201	289.76	1185247.36	758604.89									
	FH-B-202	288.08	1185316.02	758630.65									
FH-B-203	291.08	1185224.7	758591.8										
FH-B-305	253.05	1184888.39	759408.73										
FH-B-306	253.97	1184816.52	759408.82										
FH-B-307	251.77	1184906.29	759381.4										
FH-B-308	251.77	1184858	759294.24										
FH-B-309	251.60	1184917.01	759230.91										
FH-B-310	251.51	1184910.43	759225.38										
FH-B-311	251.44	1184925.8	759340.53										
FH-B-312	251.37	1184977.85	759281.94										
FH-B-6	250.49	1185243.1	759210.41										
FH-B-7	251.47	1184991.6	759088.96										
FH-B-8	250.78	1185053.7	759323.52										
Wave Surge Protection Project Army Corp April-95	D94-1	224.33	1189750	761500	0.0-2.0								GRAIN SIZE
					6.0-8.0								GRAIN SIZE
	D94-2	232.33	1188762	761356	0.0-2.0		34	25	9				GRAIN SIZE
					5.0-7.0		37	27	10				GRAIN SIZE
D94-3	238.33	1187239	760364	10.0-12.0	69.9								GRAIN SIZE LOI = 7.6%
Rehabilitation of East Pier Army Corp June-85	D79-6	238.43	1188337.56	760567.11	12.0-12.7		40	23	17				GRAIN SIZE
					16.3-17.8								GRAIN SIZE
					31.3-32.8								GRAIN SIZE

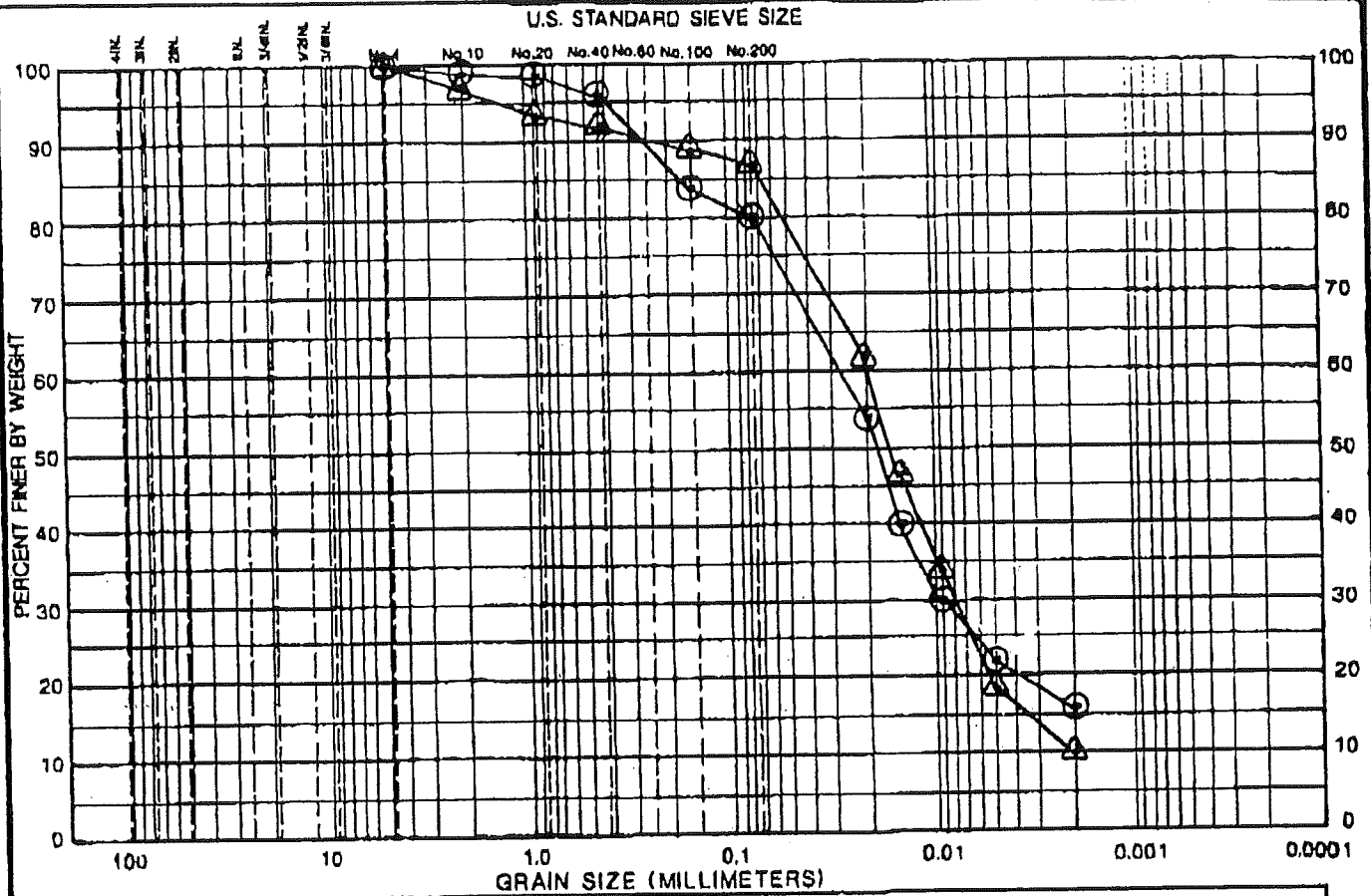
NOTES: 1. SPT and probe blow counts are recorded on the subsurface exploration logs included in Appendix ???

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APPENDIX C

Records of Earlier Subsurface Explorations

- C-1 Stutson Street Water Main**
- C-2 Genesee River Crossing**
- C-3 Stutson Street Bridge**
- C-4 Dredge Probes – Army Corp of Engineers**
- C-5 Wave Surge Protection Project**
- C-6 Rehabilitation of East Pier**
- C-7 Lake Avenue Improvements**



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APPENDIX D

Historic (Sanborn) Maps

- D-1 Site Map with 1892 Sanborn Overlay**
- D-2 Site Map with 1912 Sanborn Overlay**
- D-3 Site Map with 1924 Sanborn Overlay**
- D-4 Site Map with 1950 Sanborn Overlay**
- D-5 Site Map with 1967 Sanborn Overlay**

